# APPENDIX 3C SURFACE WATER MONITORING DATA

#### **Summary of 2024 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L)			Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	3200	4790	4138	7.7	7.9	7.7	957	1790	1411	1.3	13.0	6.7
CC2	3160	5200	4165	7.4	8.2	7.9	1160	2120	1563	0.8	15.5	4.4
CC3	2390	3720	3214	8.2	8.5	8.3	796	1560	1255	0.8	15.8	5.3
WIL (U)*	505	646	576	5.8	6.1	6.0	106	125	116	8.7	78.2	43.5
WIL (U2)	484	1320	869	6.7	7.4	6.9	8	136	45	12.9	144.0	46.0
WIL (PC)*												
WIL (NC)^												
WIL (D)	571	1730	780	7.7	8.1	7.8	57	505	139	5.1	61.7	17.2
WIL (D2)*	543	1570	771	7.7	8.0	7.9	53	449	138	3.7	27.9	7.3
WOL1	596.0	1940.0	819.5	7.9	8.3	8.1	51.0	561.0	146.8	3.6	14.5	8.8
WOL2	1120.0	2040.0	1550.9	7.6	8.1	8.0	210.0	350.0	279.3	3.6	49.3	12.8

Notes: mg/L = micrograms per litre. mS/cm= micro-Siemens per centimetre. NTU = nephelometric turbidity units. \*Dry ^ No Access

#### Summary of 2023 Surface Water Monitoring Results

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L)			Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	2220	4280	3496	7.6	7.9	7.7	630	1580	1191	1.5	66.9	17.0
CC2	2210	3980	2842	7.7	8.2	8.0	629	1610	875	0.4	55.7	7.3
CC3	1730	2450	2105	7.9	8.6	8.2	506	875	672	1.0	31.8	9.6
WIL (U)*	737	2230	1076	6.9	8.0	7.3	48	655	178	9.2	116.0	43.0
WIL (U2)	738	2240	1104	6.7	8.2	7.3	28	649	109	5.5	76.2	32.5
WIL (PC)*	2300	2300	2300	8.0	8.0	8.0	571	571	571	31.4	31.4	31.4
WIL (NC)*	448	450	449	7.3	7.4	7.4	93	99	96	<0.1	<0.1	<0.1
WIL (D)	638	1890	1269	7.6	8.4	8.1	97	778	350	3.1	22.0	7.9
WIL (D2)*	682	1790	1246	7.8	8.3	8.0	127	588	340	2.3	15.1	4.5
WOL1	718.0	1820.0	1233.6	7.8	8.6	8.2	129.0	742.0	340.4	3.8	36.2	10.4
WOL2	1080.0	1550.0	1320.8	8.0	8.6	8.2	147.0	307.0	229.1	3.0	12.0	6.6

 $Notes: mg/L = micrograms \ per \ litre. \ mS/cm = micro-Siemens \ per \ centimetre. \ NTU = nephelometric \ turbidity \ units. \ *Dry \ and \ substitute \ and \ su$ 

#### **Summary of 2022 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L)			Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	1870	3370	2917	7.6	8.0	7.8	551	1320	971	1.1	12.8	4.2
CC2	1170	4130	2465	7.7	8.2	8.0	319	1450	766	0.3	3.2	1.7
CC3	411	2060	1426	7.6	8.4	8.0	69	626	392	0.9	13.2	3.7
WIL (U)*	221	1510	667	6.9	7.6	7.2	5	448	138	7.3	24.9	14.8
WIL (U2)	210	1440	694	6.7	7.6	7.1	7	412	139	6.9	24.0	13.4
WIL (PC)*	432	1410	657	6.9	7.8	7.3	9	282	81	25.8	74.0	40.7
WIL (NC)*	396	3530	1208	7.0	8.0	7.3	34	1380	391	0.4	5.0	1.7
WIL (D)	497	3260	1418	7.5	8.3	7.9	47	1160	402	3.6	43.8	14.3
WIL (D2)*	527	2790	1410	7.6	8.0	7.9	67	917	387	2.6	12.4	7.6
WOL1	824.0	2760.0	1258.0	7.7	8.1	8.0	101.0	915.0	302.6	2.3	14.5	7.0
WOL2	609.0	1210.0	806.2	6.9	8.2	7.6	54.0	144.0	93.3	2.2	69.1	18.0

 $Notes: mg/L = micrograms \ per \ litre. \ mS/cm = micro-Siemens \ per \ centimetre. \ NTU = nephelometric \ turbidity \ units. \ *Dry \ and \ substitute \ and \ su$ 



#### **Summary of 2021 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L)			Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	179.0	4880.0	2802.8	7.0	7.9	7.6	14.0	1740.0	884.9	2.1	366.0	80.4
CC2	3080.0	7870.0	5356.4	7.8	8.2	8.0	811.0	3000.0	1938.3	0.5	2.8	1.0
CC3	2090.0	3310.0	2508.6	8.3	8.7	8.4	593.0	1130.0	756.6	0.8	18.3	7.0
WIL (U)*	258.0	511.0	391.8	6.9	7.2	7.0	6.0	52.0	24.2	7.5	19.3	12.7
WIL (U2)	321.0	582.0	425.6	6.8	7.2	7.0	10.0	28.0	19.9	8.2	18.6	12.7
WIL (PC)*	304.0	633.0	490.6	6.8	7.2	7.0	7.0	32.0	19.4	10.1	1700.0	173.5
WIL (NC)*	343.0	609.0	477.8	6.8	7.7	7.3	51.0	89.0	66.5	1.1	164.0	35.1
WIL (D)	374.0	1330.0	606.9	7.2	7.7	7.5	34.0	317.0	102.3	1.6	13.3	5.1
WIL (D2)*	400.0	1340.0	600.3	7.3	8.0	7.7	40.0	319.0	107.4	1.6	8.8	3.6
WOL1	571.0	1670.0	1003.5	7.9	8.4	8.1	63.0	293.0	153.8	1.0	12.4	3.3
WOL2	469.0	2910.0	1526.8	7.5	8.0	7.9	51.0	471.0	241.9	0.8	11.6	3.2

Notes: mg/L = micrograms per litre. mS/cm= micro-Siemens per centimetre. NTU = nephelometric turbidity units. \*Dry

#### **Summary of 2020 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L)	)		Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	262.0	1380.0	990.7	6.9	7.6	7.4	39.0	399.0	277.3	58.1	523.0	234.7
CC2	5850.0	8500.0	6786.7	7.8	8.2	8.0	2290.0	3080.0	2516.7	0.7	325.0	38.0
CC3	4330.0	4720.0	4592.5	8.5	8.6	8.5	1710.0	1960.0	1845.0	0.6	10.0	3.2
WIL (U)*												
WIL (U2)	388.0	4070.0	975.3	4.3	7.1	6.3	30.0	421.0	108.5	7.5	270.0	52.0
WIL (PC)*												
WIL (NC)*												
WIL (D)	311.0	2650.0	799.1	3.4	7.3	6.0	38.0	1150.0	250.9	5.9	30.5	20.4
WIL (D2)*												
WOL1	537.0	2420.0	1396.2	6.3	8.4	7.8	130.0	600.0	332.6	1.2	13.9	6.2
WOL2	1920.0	6740.0	2911.7	7.0	8.2	7.7	383.0	802.0	516.8	1.6	33.5	7.0

 $Notes: mg/L = micrograms \ per \ litre. \ mS/cm = micro \ Siemens \ per \ centimetre. \ NTU = nephelometric \ turbidity \ units. \ *Dry \ and \ siemens \ per \ centimetre.$ 

#### **Summary of 2019 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L			Turbidity (N	ITU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	432.0	697.0	564.5	7.3	9.1	8.2	56.0	102.0	79.0	663.0	2310.0	1486.5
CC2	3240.0	9910.0	7207.1	7.7	8.0	7.9	884.0	3760.0	2716.3	2.0	16.0	5.1
CC3	5850.0	5850.0	5850.0	7.9	7.9	7.9	2670.0	2670.0	2670.0	4.4	4.4	4.4
WIL (U)*	-	-	-	-	-	-	-	-	-	-	-	-
WIL (U2)	3840.0	5850.0	4428.3	3.6	6.3	4.2	287.0	578.0	400.3	0.9	45.0	11.2
WIL (PC)*	-	-	-	-	-	-	-	-	-	-	-	-
WIL (NC)*	-	-	-	-	-	-	-	-	-	-	-	-
WIL (D)	1440.0	6420.0	4192.9	4.0	7.4	6.7	521.0	1960.0	1273.3	9.7	95.2	44.4
WIL (D2)*	-	-	-	-	-	-	-	-	-	-	-	-
WOL1	1180.0	4780.0	2877.5	7.9	8.5	8.1	240.0	1510.0	752.5	0.8	5.2	3.3
WOL2	1690.0	5610.0	3545.8	7.0	8.2	7.5	311.0	808.0	641.4	1.7	43.7	16.1

 $Notes: mg/L = micrograms \ per \ litre. \ mS/cm = micro \ Siemens \ per \ centimetre. \ NTU = nephelometric \ turbidity \ units. \ *Dry \ and \ siemens \ per \ centimetre.$ 



#### Summary of 2018 Surface Water Monitoring Results

SW Monitoring		EC (μS/cm	)		рН			SO4 (mg/L	)	Τι	urbidity (N	TU)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	228.0	1280.0	491.7	6.70	7.60	7.23	19.0	384.0	84.2	20.0	5520.0	1321.9
CC2	364.0	7570.0	6262.4	7.60	8.10	7.92	67.0	3000.0	2379.7	1.4	499.0	57.1
CC3	40.0	40.0	40.0	7.80	7.80	7.80	4.0	4.0	4.0	141.0	141.0	141.0
WIL (U)	-	-	-	-	-	-	-	-	-	-	-	-
WIL (U2)	1790.0	4380.0	3441.8	3.50	7.40	6.03	80.0	446.0	58.5	5.1	159.0	58.5
WIL (PC)	-	-	-	-	-	-	-	-	-	-	-	-
WIL (NC)	239.0	383.0	319.1	6.70	7.50	7.28	41.0	100.0	66.3	0.4	2.8	1.4
WIL (D)	278.0	2020.0	669.7	5.20	8.00	6.92	20.0	553.0	134.7	1.3	288.0	44.3
WIL (D2)	236.0	569.0	386.3	4.20	7.80	6.84	33.0	204.0	80.9	1.6	396.0	104.3
WOL1	425.0	2150.0	1260.1	7.20	8.40	8.01	41.0	494.0	294.1	1.0	19.6	6.8
WOL2	1730.0	2850.0	2404.5	7.00	7.90	7.51	209.0	740.0	447.7	1.0	36.2	6.1

#### **Summary of 2017 Surface Water Monitoring Results**

SW Monitoring		EC (μS/cm)			рН			SO4 (mg/L	)		Turbidity (NT	·U)
Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	279.0	5380.0	2392.3	7.00	8.30	7.58	45.0	1790.0	787.0	4.4	1970.0	600.9
CC2	5470.0	8230.0	6306.0	7.70	8.30	7.99	1700.0	3170.0	2145.0	0.6	15.8	4.1
ССЗ	4100.0	4990.0	4520.0	8.30	8.50	8.40	1490.0	1920.0	1688.0	0.6	1.8	1.2
WIL (U)*	ı	-	-	ı	ı	-	-	ı	ı	-	-	-
WIL (U2)	1360.0	3890.0	2851.7	5.40	8.00	6.58	13.0	121.0	20.9	2.4	70.8	20.9
WIL (PC)*	-	-	-	-	-	-	-	-	-	-	-	-
WIL (NC)	230.0	411.0	313.2	6.80	8.30	7.27	10.0	85.0	48.1	0.2	15.2	3.7
WIL (D)	248.0	1480.0	493.5	7.30	7.80	7.55	7.0	87.0	46.4	2.2	5.6	3.8
WIL (D2)	256.0	650.0	386.8	7.30	7.90	7.53	2.0	83.0	47.7	1.7	31.9	10.3
WOL1	336.0	1490.0	872.4	8.10	8.60	8.25	19.0	184.0	97.2	0.9	6.1	2.9
WOL2	1800.0	2950.0	2133.6	7.40	8.00	7.82	184.0	440.0	304.2	0.4	21.1	3.2

Notes: mg/L = micrograms per litre. mS/cm= micro Siemens per centimetre. NTU = nephelometric turbidity units. \*Dry

#### **Summary of 2016 Surface Water Monitoring Results**

SW		EC (μS/cm)			рН			SO <sub>4</sub> (mg/L)			Turbidity (N	ΓU)
Monitoring Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	170.0	4470.0	2802.9	7.10	7.90	7.41	28.0	1710.0	978.9	4.6	6270.0	936.0
CC2	3020.0	7540.0	5036.3	7.50	8.00	7.84	920.0	2940.0	1738.8	0.5	26.4	5.0
CC3	80.0	4860.0	2771.7	7.40	8.40	8.18	8.0	1920.0	972.5	0.7	126.0	25.1
WIL (U)	520.0	950.0	632.0	6.20	7.40	6.94	13.0	83.0	36.8	5.8	43.5	21.2
WIL (U2)	440.0	4420.0	2140.0	6.50	7.60	7.04	14.0	102.0	34.8	3.3	153.0	34.8
WIL (PC)	260.0	1340.0	682.0	6.90	7.40	7.16	7.0	48.0	28.6	9.7	64.6	38.3
WIL (NC)	240.0	1650.0	560.8	7.10	7.80	7.39	8.0	265.0	64.5	8.6	201.0	54.2
WIL (D)	580.0	3030.0	1189.2	6.80	8.00	7.46	12.0	603.0	165.5	1.2	39.4	10.0
WIL (D2)	390.0	1840.0	796.1	6.90	8.10	7.50	9.0	466.0	159.1	3.9	323.0	43.8
WOL1	780.0	2220.0	1226.3	7.80	8.30	8.11	104.0	475.0	205.8	1.3	11.2	5.0
WOL2	740.0	3160.0	1693.3	7.20	8.00	7.56	97.0	650.0	303.1	0.9	70.7	15.3
SGC_1*	0	0	0	0	0	0	0	0	0	0	0	0

Notes: mg/L = micrograms per litre. mS/cm= micro Siemens per centimetre. NTU = nephelometric turbidity units. \*Dry



#### Summary of 2015 Surface Water Monitoring Results

SW		EC (μS/cm	)		рН			SO <sub>4</sub> (mg/L)		Tu	rbidity (N1	U)
Monitoring Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	120.0	4380.0	2316.3	6.60	7.80	7.31	13.0	1660.0	237.7	3.3	13000. 0	3415.4
CC2	350.0	5970.0	3591.4	7.30	7.90	7.67	1400.0	2290.0	1977.8	0.4	20.8	4.7
CC3	150.0	5130.0	2220.0	7.00	8.40	7.93	17.0	2100.0	946.0	1.2	359.0	93.7
WIL (U)	1650.0	7550.0	4306.7	4.80	6.80	5.93	38.0	146.0	99.0	7.4	263.0	77.0
WIL (U2)	790.0	5580.0	3353.8	5.60	7.40	6.71	22.0	118.0	41.9	1.5	158.0	41.9
WIL (PC)*	1170.0	6100.0	3256.3	6.80	7.90	7.23	3.0	42.0	16.0	1.8	222.0	90.4
WIL (NC)	410.0	3960.0	1987.1	6.60	7.80	7.31	4.0	106.0	43.0	1.2	1440.0	284.5
WIL (D)	340.0	5880.0	2713.0	7.10	8.10	7.67	29.0	607.0	253.2	2.6	363.0	63.1
WIL (D2)	500.0	6520.0	2457.5	7.50	8.20	7.73	16.0	693.0	148.4	7.5	557.0	113.2
WOL1	160.0	5540.0	2223.0	7.50	8.20	7.96	208.0	956.0	445.8	1.1	61.8	13.3
WOL2	400.0	5550.0	1830.0	7.30	7.80	7.54	262.0	822.0	532.8	0.6	486.0	53.9

Notes: mg/L = micrograms per litre. mS/cm= micro Siemens per centimetre. NTU = nephelometric turbidity units.

#### **Summary of 2014 Surface Water Monitoring Results**

SW		EC (μS/cm	)		рН			SO <sub>4</sub> (mg/L)		Tu	rbidity (NT	U)
Monitoring Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	610.0	5430.0	2055.7	7.10	9.20	8.00	120.0	1880.0	785.0	2.3	352.0	91.3
CC2	160.0	6590.0	4944.0	6.90	7.80	7.44	85.0	2520.0	1733.5	0.2	151.0	16.4
CC3	400.0	5260.0	3522.5	7.60	8.00	7.80	23.0	2100.0	1380.8	1.1	346.0	96.0
WIL (U)	980.0	1540.0	1260.0	6.00	7.10	6.55	70.0	174.0	122.0	3.2	30.0	16.6
WIL (U2)	1340.0	5970.0	2886.0	6.30	7.40	6.78	10.0	110.0	50.1	4.5	290.0	50.1
WIL (PC)	-	-	-	-	-	-	-	-	-	1	-	-
WIL (NC)	310.0	790.0	445.0	7.00	7.40	7.25	6.0	96.0	27.0	1.8	2410.0	664.4
WIL (D)	1520.0	6010.0	3728.3	6.90	8.40	7.68	205.0	1680.0	634.8	1.0	26.8	6.6
WIL (D2)	780.0	7550.0	3756.0	7.00	8.70	8.02	120.0	1670.0	932.4	0.8	42.7	11.7
WOL1	1870.0	3680.0	2582.5	7.00	8.90	8.13	434.0	1120.0	635.6	1.2	18.6	3.8
WOL2	1670.0	4060.0	2779.2	7.20	7.80	7.46	452.0	842.0	589.9	0.6	69.7	16.1

Notes: mg/L = micrograms per litre. mS/cm= micro Siemens per centimetre. NTU = nephelometric turbidity units. \* Indicates no sample available during the schedule monitoring programme.

#### **Summary of 2013 Surface Water Monitoring Results**

SW		EC (μS/cm	)		рН			SO <sub>4</sub> (mg/L)		Tu	rbidity (N1	U)
Monitoring Point	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave	Min	Max	Ave
CC1	3150.0	5710.0	4568.5	6.9	8.2	7.9	828.0	3160.0	1647.0	0.4	1770	169.6
CC2	4380.0	6070.0	5040.0	7.4	8.1	7.7	1610.0	3110.0	2040.0	0.2	2.6	0.9
CC3	225.0	4890.0	3130.6	7.8	8.2	8.0	94.0	2270.0	1454.1	0.8	360.0	59.4
WIL (U)	448.0	1390.0	1065.0	6.5	7.0	6.8	7.0	63.0	38.1	1.5	74.5	26.5
WIL (U2)	413.0	4620.0	2165.5	6.3	7.6	6.7	4.0	89.0	47.4	6.1	473.0	62.8
WIL (PC)	395.0	1730.0	1158.0	6.7	7.1	6.9	31.0	186.0	93.8	5.2	148.0	47.6
WIL (NC)	340.0	930.0	510.0	7.4	7.9	7.7	5.0	140.0	59.6	2.2	4000	941.5
WIL (D)	1656.0	4200.0	2942.6	7.8	8.8	8.1	216.0	822.0	475.2	1.4	59.1	9.3
WIL (D2)	1500.0	4950.0	3051.6	7.8	8.1	7.9	217.0	1360.0	646.7	1.2	21.8	7.0
WOL1	1180.0	2710.0	1982.3	8.1	8.7	8.4	326.0	675.0	464.8	0.6	8.9	3.0
WOL2	1460.0	3150.0	2153.9	7.3	8.3	7.9	286.0	793.0	487.7	0.6	14.9	6.0



### 2024 Results for Surface Water Monitoring

Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25oC µS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	Ionic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meq/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
CC_1	10-Jan- 2024	1259			<0.0 01	0.06 6	616		<1		4570	<0.0 01		1		<1		0.56	<0.0 01		1.31	<0.0 01	0.00 5	7.8				<0.0 1		2.07	1480	25.5	616				5.8	<0.0 05
CC_2	10-Jan- 2024	1008			<0.0 01	0.02	278		<1		4460	<0.0 01		1		<1		0.18	<0.0 01		1.72	0.00	0.00	7.6				<0.0		2.16	1740	23	278				13.2	<0.0 05
CC_3	10-Jan- 2024	920			- 02							02		0					- 02			_																
WIL_U	10-Jan- 2024	1348												0																								
WIL_U2	10-Jan- 2024	1414			<0.0 01	0.05 9	90		<1		1320	<0.0 01		0		<1		4.19	<0.0 01		2.54	<0.0 01	0.01 9	6.8				<0.0 1		0.36 9	136	31.5	90				30	<0.0 05
WIL_NC	10-Jan- 2024	1320												0																								
WIL_PC	10-Jan- 2024	1343												0																								
WIL_D	10-Jan- 2024	1135			<0.0 01	0.02	135		<1		767	<0.0 01		1		<1		1.19	<0.0 01		0.11 7	<0.0 01	0.00	7.7				<0.0 1		0.19 4	128	24	135				16.7	<0.0 05
WIL_D2	10-Jan- 2024	1205			<0.0 01	0.01	129		<1		733	<0.0 01		1		<1		0.81	<0.0 01		0.07 8	<0.0 01	0.00	7.7				<0.0 1		0.16 9	124	25	129				7.3	<0.0 05
WOL_1	10-Jan- 2024	1051			<0.0 01	0.02	134		<1		776	<0.0 01		1		<1		0.87	<0.0 01		0.06 2	<0.0 01	0.00	7.9				<0.0 1		0.2	131	25	134				13.4	<0.0 05
WOL_2	10-Jan- 2024	1110			<0.0 01	0.04 6	217		<1		1560	<0.0 01		1		<1		1.03	<0.0 01		0.83 8	<0.0 01	0.00	7.6				<0.0 1		0.74 5	324	23	217				14	<0.0 05
SGC_1	10-Jan- 2024	1500												0																								
CC_1	07-Feb- 2024	1212	16	0.09	0.00	0.07 2	594	177	<1	382	3990	<0.0 01	5.5	1	13	<1	0.72	0.86	<0.0 01	213	1.78	0.00 1	0.00 6	7.7	7.7	42	124	<0.0 1	491	1.63	1290	19	594	49.5	535	48.8	13	<0.0 05
CC_2	07-Feb- 2024	1535	12	0.17	<0.0 01	0.02 3	209	398	<1	507	4630	0.00	7.6	1	4	<1	3.7	0.44	<0.0 01	249	0.37 3	<0.0 01	0.00 5	7.4	7.37	22	128	<0.0 1	396	2.14	2120	20.5	209	62.6	188	58.1	5.1	<0.0 05
CC_3	07-Feb- 2024	1554												0																								
WIL_U	07-Feb- 2024	1126												0																								
WIL_U2	07-Feb- 2024	1046	6	0.37	<0.0 01	0.03 2	47	16	<1	94	484	0.00	8.4	1	5	<1	1.9	3.2	<0.0 01	15	0.41 8	<0.0 01	0.01 1	6.8	6.87	10	127	<0.0 1	56	0.11 4	46	20.5	47	4.55	46	4.72	144	0.00 8
WIL_NC	07-Feb- 2024	1156																																				
WIL_PC	07-Feb- 2024	1121												0																								
WIL_D	07-Feb- 2024	1353	3	0.18	<0.0 01	0.01	148	23	<1	86	742	<0.0 01	9.1	1	2	<1	0.68	1.15	<0.0 01	21	0.09	<0.0 01	0.00	7.7	7.73	11	135	<0.0	100	0.16	107	21	148	7.61	132	7.51	15.5	<0.0 05
WIL_D2	07-Feb- 2024	1313	3	0.15	<0.0 01	0.01	140	19	<1	79	687	0.00	9.2	1	2	<1	0.43	0.99	<0.0 01	19	0.09	<0.0 01	0.00	7.7	7.71	10	138	<0.0	96	0.13 9	95	21	140	7	125	6.94	7.7	<0.0 05
WOL_1	07-Feb- 2024	1431	2	0.19	<0.0 01	0.01 9	154	20	<1	82	683	0.00	9.3	1	2	<1	2.33	1.21	<0.0 01	17	0.04	<0.0 01	0.00	7.9	7.83	8	138	<0.0	95	0.14	80	22	154	7.06	137	6.73	14.5	<0.0 05
WOL_2	07-Feb- 2024	1456	3	0.12	0.00	0.05 5	588	71	<1	254	1700	<0.0 01	7.9	0	8	<1	13.7	0.68	<0.0 01	82	0.56	0.00	0.00	8	7.92	7	142	<0.0 1	188	0.68 2	272	23	588	24.6	526	18.6	27.1	<0.0 05
SGC_1	07-Feb- 2024	1500																				_													<u> </u>	<u> </u>		
CC_1	06-Mar- 2024	1047	7	0.04	0.00	0.07	731		<1		3910	<0.0 01		0		<1		0.48	<0.0 01		0.60 8	<0.0 01	0.00 4	7.8				<0.0 1		1.66	957	18.5	731		 	<u> </u>	6.2	<0.0 05
CC_2	06-Mar- 2024	1352												0																					<u> </u>	<u> </u>	$\sqcup$	
CC_3	06-Mar- 2024	1410												0																					, <u> </u>	<u> </u>		
WIL_U	06-Mar- 2024	1011		.0.0		201								0							0.00		0.00							0.00					ļļ	<u> </u>		
WIL_U2	06-Mar- 2024	937	9	<0.0 1	<0.0 01	0.01 8	166		<1		1020	<0.0 01		0		<1		2.96	<0.0 01		0.80 1	<0.0 01	0.00 5	7.1				<0.0 1		0.28 2	20	20.5	166				26.6	<0.0 05



Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25oC µS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	lonic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meq/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
WIL_NC	06-Mar- 2024	1024																																				
WIL_PC	06-Mar- 2024	1007												0																								
WIL_D	06-Mar- 2024	1218	10	0.18	<0.0 01	0.01 3	125		<1		571	<0.0 01		1		<1		0.83	<0.0 01		0.04 4	<0.0 01	0.00 2	7.7				<0.0 1		0.11 5	57	22.5	125				9.8	<0.0 05
WIL_D2	06-Mar- 2024	1122	11	0.1	<0.0 01	0.00 8	120		<1		550	<0.0 01		1		<1		0.91	<0.0 01		0.04 7	<0.0 01	0.00 2	8				<0.0 1		0.10 4	53	22	120				6.3	<0.0 05
WOL_1	06-Mar- 2024	1248	9	0.14	<0.0 01	0.01 4	132		<1		596	<0.0 01		1		<1		0.68	<0.0 01		0.02 4	<0.0 01	0.00 2	8.2				<0.0 1		0.12 4	58	24	132				8.7	<0.0 05
WOL_2	06-Mar- 2024	1315	9	0.17	0.00 1	0.06 4	311		<1		2010	<0.0 01		0		<1		0.81	<0.0 01		0.37	0.00	0.00 2	8				<0.0 1		0.85 8	303	25	311				15.7	<0.0 05
SGC_1	06-Mar- 2024	1230																																				
CC_1	10-Apr- 2024	1038	18	0.05	0.00 1	0.07 2	655		<1		4210	<0.0 01		1		<1		0.56	<0.0 01		0.73 2	<0.0 01	0.00 4	7.9				<0.0 1		1.81	1300	13	655				6	<0.0 05
CC_2	10-Apr- 2024	1546	18	<0.0 1	<0.0 01	0.02 2	337		<1		5200	<0.0 01		1		<1		<0.0 5	<0.0 01		0.27 3	0.00	0.00	7.6				<0.0 1		2.48	1930	16.5	337				0.8	<0.0 05
CC_3	10-Apr- 2024	1625												0																								
WIL_U	10-Apr- 2024	1256												0																								
WIL_U2	10-Apr- 2024	1241	5	0.06	0.00 1	0.03 6	128		<1		1000	<0.0 01		1		<1		2.92	<0.0 01		0.40 9	<0.0 01	0.00 6	7.4				<0.0 1		0.25 2	34	21	128				24.6	<0.0 05
WIL_NC	10-Apr- 2024	1325																																				
WIL_PC	10-Apr- 2024	1251												0																								
WIL_D	10-Apr- 2024	1345	2	0.22	<0.0 01	0.01 4	144		<1		645	<0.0 01		1		<1		0.78	<0.0 01		0.03	<0.0 01	0.00	7.8				<0.0		0.13	96	18	144				8.2	<0.0 05
WIL_D2	10-Apr- 2024	942	2	0.13	<0.0 01	0.01	145		<1		655	<0.0 01		1		<1		0.89	<0.0 01		0.06	<0.0 01	0.00	7.7				<0.0		0.13	92	14.5	145				5.8	<0.0 05
WOL_1	10-Apr- 2024	1428	2	0.23	<0.0 01	0.01 8	147		<1		637	<0.0 01		1		<1		0.63	<0.0		0.04	01	0.00	8				<0.0		0.13	51	17.5	147				10.6	<0.0 05
WOL_2	10-Apr- 2024	1511	<1	0.13	<0.0 01	0.04 7	202		2		1200	<0.0 01		1		<1		0.58	<0.0 01		0.10 7	<0.0 01	0.00	7.9				<0.0 1		0.59 2	213	17	204				8.5	<0.0 05
SGC_1	10-Apr- 2024	1410			0.00	0.07						10.0							10.0			10.0	0.00					10.0										10.0
CC_1	08-May- 2024	1112	4	0.04	0.00	0.07	641	250	11	464	4790	<0.0 01	7.7	1	6	<1	1.26	0.72	<0.0	264	1.09	<0.0 01	0.00	7.7	7.75	35	162	<0.0	599	2.16	1610	13	652	59.6	575	61.2	10.4	<0.0 05
CC_2	08-May- 2024	1501	<1	0.03	<0.0 01	0.02 5	323	343	<1	492	4470	<0.0 01	9.1	1	3	<1	1.57	<0.0 5	<0.0 01	250	0.33 5	0.00	0.00	7.9	7.83	30	160	<0.0 1	423	2.23	1670	17.5	323	55.1	288	56.8	1.6	<0.0 05
CC_3	08-May- 2024	1550												0																								
WIL_U	08-May- 2024 08-May-	1213			<0.0	0.03						<0.0		0					<0.0		0.54	<0.0	0.00					<0.0		0.24								<0.0
WIL_U2	2024 08-May-	1142	3	0.03	01	3	126	22	<1	238	1020	0.0	9.7	0	2	<1	0.71	2.08	01	29	1	01	9	7.2	7.43	7	143	1	142	7	36	16	126	9.98	113	9.84	12.9	0.0
WIL_NC	2024 08-May-	1230																																				
WIL_PC	2024 08-May-	1210			<0.0	0.01						<0.0		0					<0.0		0.03	<0.0	0.00					<0.0		0.15								<0.0
WIL_D	2024 08-May-	1308	<1	0.12	0.0 01 <0.0	0.01	144	19	<1	66	690	01 <0.0	10	1	2	<1	1.68	0.61	0.0 01 <0.0	20	0.03	01	0.00	7.8	7.87	6	141	1 <0.0	99	4	100	15	144	6.82	129	7.05	7	0.0 05 <0.0
WIL_D2	2024 08-May-	1013	<1	<0.0	01 <0.0	2 0.02	156	27	<1	78	843	01 <0.0	10.3	1	2	<1	1.9	0.36 <0.0	0.0	28	7 <0.0	01	1	7.8	7.93	7	148	1 <0.0	114	0.2	151	13.5	156	8.46	139	8.79	3.7	05 <0.0
WOL_1	2024 08-May-	1400	<1	1	0.0 01 <0.0	6 0.04	152	22	<1	71	752	01 <0.0	10.4	1	2	<1	3.83	5	0.0	23	0.06		0.00 1 0.00	8.2	8.09	7	152	1 <0.0	107	0.08 1 0.56	106	16.5	152	7.25	136	7.82	4.1	05
WOL_2	2024	1423	<1	0.15	0.0	1	190	59	<1	177	1240	0.0	10.3	1	2	<1	0.48	0.48	01	57	3	1	2	8	7.96	7	160	1	120	4	210	15	190	13.2	169	13	6.6	0.0



Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25oC µS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	Ionic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meq/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
SGC_1	08-May- 2024	1500																																				
CC_1	04-Jun- 2024	1044	9	0.01	<0.0 01	0.04 4	201		<1		3200	<0.0 01		1		<1		0.07	<0.0 01		0.04 5	<0.0 01	0.00	7.7				<0.0 1		1.29	1160	9.5	201				1.3	<0.0 05
CC_2	04-Jun- 2024	1413	6	0.01	<0.0 01	0.02	217		<1		3160	<0.0 01		1		<1		0.05	<0.0 01		0.34	<0.0 01	0.00	8				<0.0		1.26	1160	11.5	217				1.2	<0.0 05
CC_3	04-Jun- 2024	1458	9	0.01	<0.0 01	0.01 8	170		<1		2390	<0.0 01		1		<1		<0.0 5	<0.0 01		0.01	<0.0 01	0.00	8.2				<0.0 1		0.89 8	796	10.5	170				0.8	<0.0 05
WIL_U	04-Jun- 2024	1151	9	0.29	<0.0 01	0.03 9	10		<1		505	<0.0 01		0		<1		0.55	<0.0 01		0.10 9	<0.0 01	0.02	6.1				<0.0		0.11 9	125	10.5	10				78.2	0.03
WIL_U2	04-Jun- 2024	1115	18	0.13	<0.0 01	0.03	74		<1		600	<0.0 01		1		<1		9.48	<0.0 01		0.79	<0.0 01	0.00	6.8				<0.0		0.12	48	9.5	74				86.8	0.00
WIL_NC	04-Jun- 2024	1214																	-																			
WIL_PC	04-Jun- 2024	1140												0																								
WIL_D	04-Jun- 2024	1242	4	0.08	<0.0 01	0.03 4	176		<1		1730	<0.0 01		1		<1		0.64	<0.0 01		0.08 7	<0.0 01	0.00	8				<0.0 1		0.59	505	10	176				5.1	<0.0 05
WIL_D2	04-Jun- 2024	956	4	0.09	<0.0 01	0.02	173		<1		1570	<0.0 01		1		<1		0.48	<0.0 01		0.09	<0.0 01	0.00	8				<0.0		0.49	449	9	173				3.9	<0.0 05
WOL_1	04-Jun- 2024	1310	4	0.06	<0.0 01	0.05	186		<1		1940	<0.0 01		1		<1		0.42	<0.0 01		0.03	<0.0 01	0.00	8.2				<0.0		0.70 1	561	10	186				3.6	<0.0 05
WOL_2	04-Jun- 2024	1339	4	0.59	<0.0 01	0.04	151		<1		1120	<0.0 01		1		<1		1.72	<0.0 01		0.05	<0.0 01	0.00	7.9				<0.0		0.47	216	9.5	151				49.3	<0.0 05
SGC_1	04-Jun- 2024	1500																																				
CC_1	08-Jul- 2024	1055	21	0.05	0.00 1	0.06 9	464		<1		4560	<1		1		<1		1.54	<0.0 01		1.14	<0.0 01	0.00 5	7.7				<0.0 1		2.11	1790	10	464				10.5	<0.0 05
CC_2	08-Jul- 2024	1425	6	<0.0 1	<0.0 01	0.02	362		<1		3900	<1		1		<1		<0.0 5	<0.0 01		0.58	<0.0 01	0.00	8.1				<0.0		1.56	1470	12.5	362				1.7	<0.0 05
CC_3	08-Jul- 2024	1519	<1	0.14	<0.0 01	0.02 6	264		11		3470	11		1		<1		1.59	<0.0 01		0.03 9	0.00	0.00 4	8.4				<0.0 1		1.36	1460	12	275				15.8	<0.0 05
WIL_U	08-Jul- 2024	1159												0																								
WIL_U2	08-Jul- 2024	1135	13	<0.0 1	<0.0 01	0.05 2	97		<1		1010	<1		0		<1		9.7	<0.0 01		1.7	<0.0 01	0.00	6.8				<0.0 1		0.22	47	12	97				56.4	<0.0 05
WIL_NC	08-Jul- 2024	1215																																				
WIL_PC	08-Jul- 2024	1156												0																								
WIL_D	08-Jul- 2024	1250	2	0.1	<0.0 01	0.01 4	153		<1		709	<1		1		<1		0.73	<0.0 01		0.03	<0.0 01	0.00	7.9				<0.0 1		0.13 4	105	11	153				7.8	<0.0 05
WIL_D2	08-Jul- 2024	1012	2	0.06	<0.0 01	0.00 9	152		<1		712	<1		1		<1		0.5	<0.0 01		0.02	<0.0 01	<0.0 01	7.9				<0.0 1		0.13	111	10.5	152				4.3	<0.0 05
WOL_1	08-Jul- 2024	1328	2	0.07	<0.0 01	0.01 9	166		<1		881	<1		1		<1		0.49	<0.0 01		0.01 4	<0.0 01	0.00	8.2				<0.0 1		0.19 1	162	10.5	166				5.6	<0.0 05
WOL_2	08-Jul- 2024	1348	3	<0.0 1	<0.0 01	0.04 6	200		<1		1500	<1		1		<1		0.24	<0.0 01		0.03 5	<0.0 01	<0.0 01	8				<0.0 1		0.64 8	292	9.5	200				3.6	<0.0 05
SGC_1	08-Jul- 2024	1500																																				
WIL_D	16-Jul- 2024	1359	6	0.16	<0.0 01	0.01 7	166		<1		816	<1		1		<1		0.7	<0.0 01		0.04	<0.0 01	0.00	8.1				<0.0 1		0.22 1	143	9	166				7.8	<0.0 05
WIL_D2	16-Jul- 2024	1437	5	0.1	<0.0 01	0.01	151		<1		723	<1		1		<1		0.58	<0.0 01		0.03	<0.0 01	0.00	8.2				<0.0 1		0.18 7	122	9	151				3.9	<0.0 05
WILPCUMBO_CO N	16-Jul- 2024	1501	3	0.12	<0.0 01	0.01	150		<1		732	<1		1		<1		0.69	<0.0 01		0.09	<0.0 01	0.00	8.2				<0.0 1		0.18 6	144	9	150				6	<0.0 05
CC_1	14-Aug- 2024	1141	18	0.07	<0.0 01	0.04	326	264	<1	364	3700	<0.0 01	8.6	1	14	<1	0.36	0.18	<0.0 01	198	0.07 2	<0.0 01	0.00	7.7	7.73	26	84.4	<0.0 1	346	1.43	1380	12.5	326	45.5	300	45.2	3	<0.0 05
CC_2	14-Aug- 2024	1547	3	0.14	<0.0 01	0.02 6	331	272	<1	374	3650	<0.0 01	9.7	1	6	<1	1.46	0.19	<0.0 01	188	1.06	<0.0 01	0.00 4	8.2	8.06	22	92.5	<0.0 1	330	1.38	1350	17.5	331	45.3	297	44	15.5	<0.0 05



Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25οC μS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	lonic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meq/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
CC_3	14-Aug- 2024	1643	<1	0.11	<0.0 01	0.02 3	280	260	<1	270	3180	<0.0 01	10.1	1	4	<1	0.29	0.11	<0.0 01	171	0.07 1	<0.0 01	0.00	8.3	8.18	20	92	<0.0 1	264	1.15	1230	16.5	280	38.8	250	39	5.5	<0.0 05
WIL_U	14-Aug- 2024	1247	7	0.14	<0.0 01	0.03 2	6	22	<1	110	646	<0.0 01	8.6	1	12	<1	6.77	2.51	<0.0 01	20	0.75	<0.0 01	0.04 7	5.8	5.75	10	137	<0.0 1	74	0.12 7	106	12	6	5.43	17	6.22	8.7	0.04 3
WIL_U2	14-Aug- 2024	1210	5	0.19	<0.0 01	0.04	29	18	<1	107	642	<0.0 01	9.4	1	19	<1	7.8	2.75	<0.0 01	18	1.1	<0.0 01	0.01	6.7	6.56	8	141	<0.0	72	0.13	62	15	29	4.89	44	5.72	13.4	0.00
WIL_NC	14-Aug- 2024	1312			- 01	J						- 02							- 01			01	_															
WIL_PC	14-Aug- 2024	1233												0																							1	
WIL_D	14-Aug- 2024	1349	2	0.84	<0.0 01	0.02	177	36	<1	68	969	<0.0 01	10	1	6	<1	1.87	1.48	<0.0 01	31	0.08	<0.0 01	0.00	7.9	7.85	7	134	<0.0	126	0.22	201	15	177	9.64	162	10	41.3	<0.0 05
WIL_D2	14-Aug- 2024	1028	2	0.09	<0.0 01	0.01	179	33	<1	62	908	<0.0 01	9.7	1	6	<1	4.48	0.51	<0.0 01	29	0.03	<0.0 01	0.00	7.9	7.87	7	134	<0.0	124	0.20	166	14	179	8.78	163	9.61	4.5	<0.0 05
WOL_1	14-Aug- 2024	1434	1	0.11	<0.0 01	0.01	179	33	<1	66	925	<0.0 01	10	1	4	<1	2.03	0.48	<0.0 01	28	0.03	<0.0 01	0.00	8.1	8.04	7	136	<0.0	125	0.20	180	15	179	9.18	162	9.57	7.4	<0.0
WOL_2	14-Aug- 2024	1500	3	0.11	<0.0 01	0.05	213	73	<1	221	1590	<0.0 01	10.1	1	5	<1	0.84	0.45	<0.0 01	72	0.05	<0.0 01	0.00	8	7.95	8	143	<0.0	164	0.67	322	14	213	17.2	192	16.9	9.2	<0.0 05 <0.0 05
SGC_1	14-Aug- 2024	1500			01	7						01							01		-	01	1							3							1	
WILPCUMBO_CO	14-Aug- 2024	1114	2	0.12	<0.0 01	0.01	172	32	<1	60	886	<0.0 01	9.7	1	5	<1	1.98	0.56	<0.0 01	28	0.06	<0.0 01	0.00	7.9	7.92	7	136	<0.0	121	0.19	185	14	172	8.98	156	9.34	4.2	<0.0 05
N WIL_D	28-Aug-	1059	4	0.11	<0.0	0.03	222	98	<1	157	1810	<0.0	10	1	4	<1	0.33	0.47	<0.0	77	0.12	<0.0	0.00	8	7.94	12	157	<0.0	178	0.68	494	14	222	19.1	200	19.3	5.3	0.00
WIL_D2	2024 28-Aug-	1003	3	0.06	01 <0.0	0.02	206	78	<1	139	1530	<0.0	9.9	1	4	<1	3.08	0.43	<0.0	61	0.08	0.00	0.00	8	7.98	10	156	<0.0	156	0.53	429	13.5	206	17	185	16	4.1	8 <0.0 05
WILPCUMBO_CO	2024 28-Aug-	1029	2	0.05	01 <0.0	0.01	189	66	<1	120	1380	01 <0.0	9.8	1	4	<1	5.4	0.4	01 <0.0	54	0.1	<0.0	0.00	8	8.01	9	173	<0.0	142	0.41	413	15	189	15.8	170	14.1	3.6	<0.0
N CC_1	2024 10-Sep-	1136	8	0.08	01 <0.0	0.06	501		<1		4370	<0.0		1		<1		0.61	01 <0.0		0.62	01 <0.0	0.00	7.7				<0.0		1.97	1660	12	501				3.4	05 <0.0 05
CC_2	2024 10-Sep-	1506	<1	0.05	01 <0.0	0.03	382		<1		3800	<0.0		1		<1		0.07	01 <0.0		1.13	01 <0.0	0.00	8.1				<0.0		1.66	1330	20.5	382				1.3	<0.0 05
CC_3	2024 10-Sep-	1602	<1	0.15	01 <0.0	0.03			<1		3720	<0.0		1		<1		0.13	01 <0.0		0.03	0.00	0.00	8.5				<0.0		1.52	1560	20.5					3.3	<0.0
WIL_U	2024 10-Sep-	1241			01							01		0					01		8	2	2					1										05
WIL_U2	2024 10-Sep-	1210	6	0.01	<0.0	0.03	94		<1		838	<0.0		0		<1		4.62	<0.0		1.12		0.00	6.8				<0.0		0.18	34	16	94				35.2	<0.0
SGC_1	2024 10-Sep-	1500			01	9						01							01			01	7					1		1								05
WILPCUMBO_CO	2024 10-Sep-	1105	<1	0.07	<0.0	0.00	171		<1		641	<0.0		1		<1		0.49	<0.0		0.04		0.00	8.1				<0.0		0.12	106	15.5	171				3.5	<0.0
N WIL_D	2024 23-Sep-	1235	<1	0.13	<0.0	0.01	177		<1		657	<0.0		1		<1		0.67	<0.0		0.04	<0.0		8				<0.0		0.15	93	17.5	177					0.01
WIL_D2	2024 23-Sep-	1145	<1	0.07	01 <0.0	0.00	171		<1		625	01 <0.0		1		<1		0.47	01 <0.0		0.02	01 <0.0	0.00	8.1				<0.0		0.12	106	16	171					0.00
WILPCUMBO_CO	2024 23-Sep-	1207	<1	0.16	01 <0.0	9 0.01	167		<1		655	01 <0.0		1		<1		1.24	01 <0.0		7 0.20	01 <0.0		8.1				<0.0		7 0.14	117	17.5	167				7.3	<0.0
N CC_1	2024 16-Oct-	1133	11	0.16	<0.0	0.04	421		<1		4080	<0.0		1		<1		0.37	<0.0		0.39	<0.0	0.00	7.8				<0.0		1.65	1320	13	421				10.3	<0.0
CC_2	2024 16-Oct-	1514	8	0.08	01 <0.0	0.03	318		<1		3890	<0.0		1		<1		0.12	<0.0		0.80	<0.0	0.00	8				<0.0		1.74	1300	15	318				4.4	<0.0
CC_3	2024 16-Oct-	1604	<1	0.01	01 <0.0	0.02	281		<1		3310	<0.0		1		<1		<0.0	<0.0		0.01	<0.0	0.00	8.3				<0.0		1.29	1230	15.5	281				1	<0.0
WIL_U	2024 16-Oct-	1034			01	4						01		1				5	01		8	01	1					1									-+	05
WIL_U2	2024 16-Oct-	955	10	0.01	<0.0	0.04	108		<1		895	<0.0		1		<1		9.23	<0.0		1.11	<0.0	0.00	6.9				<0.0		0.20	21	14	108				44.4	<0.0
WIL_NC	2024 16-Oct-	1105	-		01							01		1					01		_	01	5					1		8							-+	05
	2024																																					



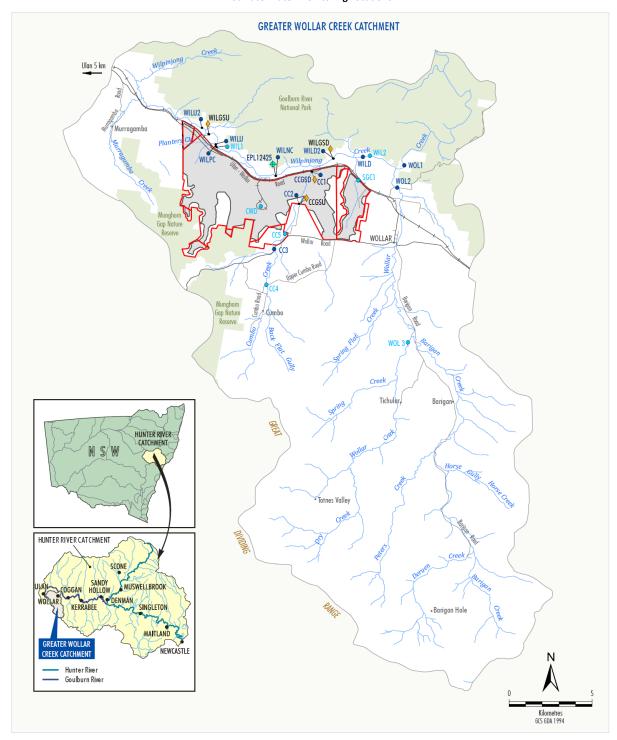
Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25oC µS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	lonic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meq/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
WIL_PC	16-Oct- 2024	1028												0																								
WIL_D	16-Oct- 2024	1335	3	1.76	<0.0 01	0.02 9	162		<1		578	<0.0 01		1		<1		3.08	0.00		0.29 4	<0.0 01	0.00 4	7.7				<0.0 1		0.12 6	67	14.5	162				61.7	<0.0 05
WIL_D2	16-Oct- 2024	1203	2	0.51	<0.0 01	0.01 5	181		<1		743	<0.0 01		1		<1		0.98	<0.0 01		0.06	<0.0 01	0.00 2	7.9				<0.0 1		0.16	118	14	181				27.9	<0.0 05
WOL_1	16-Oct- 2024	1430	<1	0.22	<0.0 01	0.01	175		<1		617	<0.0 01		1		<1		0.67	<0.0 01		0.04 6	<0.0 01	0.00	8.1				<0.0 1		0.12 3	76	15	175				9.1	<0.0 05
WOL_2	16-Oct- 2024	1447	3	0.12	<0.0 01	0.05 6	240		<1		1680	<0.0 01		1		<1		0.49	<0.0 01		0.14 2	<0.0 01	0.00 1	8				<0.0 1		0.82	312	14.2	240				5.4	<0.0 05
SGC_1	16-Oct- 2024	1500																																				
WILPCUMBO_CO N	16-Oct- 2024	1301	2	0.18	<0.0 01	0.01 6	177		<1		815	<0.0 01		1		<1		0.85	<0.0 01		0.17 2	<0.0 01	0.00	7.9				<0.0 1		0.19 8	150	14	177				8.4	<0.0 05
WIL_D	30-Oct- 2024	1046	3	0.22	<0.0 01	0.01 6	178		<1		697	<0.0 01		1		<1		0.76	<0.0 01		0.06 6	<0.0 01	0.00	7.9				<0.0 1		0.16 9	116	18.5	178				16.4	<0.0 05
WIL_D2	30-Oct- 2024	1128	1	0.07	<0.0 01	0.00 9	158		<1		601	<0.0 01		1		<1		0.52	<0.0 01		0.03	<0.0 01	0.00	8.1				<0.0 1		0.12 9	56	20	158				5.4	<0.0 05
WILPCUMBO_CO N	30-Oct- 2024	1154	1	0.07	<0.0 01	0.01	154		<1		609	<0.0 01		1		<1		0.53	<0.0 01		0.06 9	<0.0 01	0.00 1	8.1				<0.0 1		0.12 8	99	21.5	154				4.2	<0.0 05
CC_1	13-Nov- 2024	1113												0																								
CC_2	13-Nov- 2024	1444	18	0.02	0.00 1	0.07 6	376	322	<1	583	4800	<0.0 01	8.5	1	9	<1	2.14	0.73	<0.0 01	262	3.29	<0.0 01	0.00 4	7.7	7.75	16	337	<0.0 1	465	2.18	1770	23.5	376	60.8	340	58.3	2.3	0.00
CC_3	13-Nov- 2024	1515												0																								
WIL_U	13-Nov- 2024	1035												0																								
WIL_U2	13-Nov- 2024	956												0																								
WIL_NC	13-Nov- 2024	1101												1																								
WIL_PC	13-Nov- 2024	1031												0																								
WIL_D	13-Nov- 2024	1300	2	0.2	<0.0 01	0.01 3	90	14	<1	35	579	<0.0 01	9	1	23	<1	7.71	0.9	<0.0 01	14	0.06 8	<0.0 01	0.00	7.9	7.73	5	324	<0.0 1	84	0.12 7	98	23	90	4.82	102	5.63	12.5	<0.0 05
WIL_D2	13-Nov- 2024	1142	2	0.08	<0.0 01	0.00	124	13	<1	34	543	<0.0 01	9.2	1	2	<1	1.68	0.62	<0.0 01	14	0.03 7	<0.0 01	0.00 2	8	7.8	4	323	<0.0 1	82	0.10 8	89	21.5	124	5.29	111	5.47	4.8	<0.0 05
WOL_1	13-Nov- 2024	1345	2	0.36	<0.0 01	0.01 5	143	16	<1	41	611	<0.0 01	9.2	1	2	<1	1.4	0.89	<0.0 01	16	0.06 2	<0.0 01	0.00	8.2	7.99	6	317	<0.0 1	89	0.13 8	94	24	143	5.97	128	6.14	11.6	<0.0 05
WOL_2	13-Nov- 2024	1407	6	0.06	<0.0 01	0.06 7	306	94	<1	295	2040	<0.0 01	8.9	1	5	<1	0.55	0.54	<0.0 01	93	0.25	<0.0 01	0.00 2	7.9	7.86	7	322	<0.0 1	206	1.02	350	23	306	21.7	274	21.5	5	<0.0 05
SGC_1	13-Nov- 2024	1500																																				
WILPCUMBO_CO N	13-Nov- 2024	1218	2	0.07	<0.0 01	0.01	136	13	<1	35	564	<0.0 01	9.3	1	2	<1	3.07	0.58	<0.0 01	14	0.06	<0.0 01	0.00 2	8	7.83	4	313	<0.0 1	80	0.11 6	97	23.5	136	5.72	122	5.38	3.8	<0.0 05
WIL_D	25-Nov- 2024	1036	9	0.22	<0.0 01	0.01 4	157	18	<1	38	614	<0.0 01	8.4	1	2	<1	3.32	0.94	<0.0 01	15	0.08 6	<0.0 01	0.00	7.8	7.63	5	224	<0.0 1	82	0.14 9	97	23	157	6.23	140	5.83	17.1	<0.0 05
WIL_D2	25-Nov- 2024	1105	4	0.14	<0.0 01	0.01	150	18	<1	39	624	<0.0 01	8.9	1	2	<1	3.48	0.72	<0.0 01	16	0.06	0.00	0.00	7.9	7.79	5	219	<0.0 1	84	0.14 9	112	24	150	6.43	134	6	5.8	<0.0 05
WILPCUMBO_CO N	25-Nov- 2024	1122	4	0.09	<0.0 01	0.01	141	18	<1	37	614	<0.0 01	9.8	1	2	<1	1.17	0.69	<0.0 01	16	0.09 4	<0.0 01	0.00	7.9	7.72	5	219	<0.0 1	81	0.14 3	103	24	141	6	126	5.87	6.2	0.01
CC_1	10-Dec- 2024	1059	22	0.03	0.00	0.05	457		<1		4140	<0.0 01		1		<1		0.45	<0.0 01		2.82	0.00	0.00	7.7				<0.0		1.88	1570	22	457			<u> </u>	3.6	<0.0 05
CC_2	10-Dec- 2024	1428	14	0.01	0.00	0.03 8	402		<1		3850	<0.0 01		1		<1		0.09	<0.0 01		0.47 6	<0.0 01	0.00	7.8				<0.0 1		1.63	1350	28	402			<u> </u>	1	<0.0 05
CC_3	10-Dec- 2024	1521												0																						<u> </u>		
WIL_U	10-Dec- 2024	1013												1																								



Sample Location	Sampling Date	Sampling Time	Acidity as CaCO3 mg/L	Aluminium mg/L	Arsenic mg/L	Barium mg/L	Bicarbonate Alkalinity as CaCO3 mg/L	Calcium - Dissolved mg/L	Carbonate Alkalinity as CaCO3 mg/L	Chloride mg/L	Conductivity @ 25οC μS/cm	Copper mg/L	Dissolved Oxygen - Dissolved mg/L	Flow Rate	Free Carbon Dioxide as CO2 mg/L	Hydroxide Alkalinity as CaCO3 mg/L	Ionic Balance %	Iron mg/L	Lead mg/L	Magnesium - Dissolved mg/L	Manganese mg/L	Molybdenum mg/L	Nickel mg/L	pH pH Unit	pH Redox pH Unit	Potassium - Dissolved mg/L	Redox Potential mV	Selenium mg/L	Sodium - Dissolved mg/L	Strontium mg/L	Sulfate as SO4 - Turbidimetric mg/L	Temperature °C	Total Alkalinity as CaCO3 mg/L	Total Anions meg/L	Total Carbon Dioxide as CO2 mg/L	Total Cations meq/L	Turbidity NTU	Zinc mg/L
WIL_U2	10-Dec- 2024	944	17	0.03	<0.0 01	0.04 3	124		<1		726	<0.0 01		1		<1		6.16	<0.0 01		2.62	<0.0 01	0.01 2	6.9				<0.0 1		0.20 6	8	24.5	124				31.2	<0.0 05
WIL_NC	10-Dec- 2024	1036												1																								
WIL_PC	10-Dec- 2024	1009												0																								
WIL_D	10-Dec- 2024	1239	3	0.25	<0.0 01	0.01 5	146		<1		683	<0.0 01		1		<1		0.89	<0.0 01		0.06 2	<0.0 01	0.00 2	7.8				<0.0 1		0.17 7	122	26.5	146				10.6	<0.0 05
WIL_D2	10-Dec- 2024	1124	2	0.06	<0.0 01	0.01 1	139		<1		668	<0.0 01		1		<1		0.65	<0.0 01		0.06 4	<0.0 01	0.00 2	8				<0.0 1		0.16 4	125	25	139				5.2	<0.0 05
WOL_1	10-Dec- 2024	1323	2	0.3	<0.0 01	0.01 8	160		<1		758	<0.0 01		1		<1		0.76	<0.0 01		0.08 1	<0.0 01	0.00	8.1				<0.0 1		0.19 4	145	27	160				9.2	<0.0 05
WOL_2	10-Dec- 2024	1354	3	0.07	<0.0 01	0.05	243		<1		1320	<0.0 01		1		<1		0.48	<0.0 01		0.12	0.00 1	0.00 2	8				<0.0 1		0.65 3	224	26	243				5.4	<0.0 05
SGC_1	10-Dec- 2024	1500																																				
WILPCUMBO_CO N	10-Dec- 2024	1159	2	0.06	<0.0 01	0.01 1	138		<1		673	<0.0 01		1		<1		0.76	<0.0 01		0.06 5	<0.0 01	0.00 2	7.9				<0.0 1		0.16 8	130	26	138				5.3	<0.0 05
WIL_D	23-Dec- 2024	1205	2	0.33	<0.0 01	0.01	156		<1		546	<0.0 01		1		<1		0.94	<0.0 01		0.05	<0.0 01	0.00	7.8				<0.0 1		0.09 5	91	23	156				13.3	<0.0 05
WIL_D2	23-Dec- 2024	1100	2	0.08	<0.0 01	0.00 6	149		<1		549	<0.0 01		1		<1		0.73	<0.0 01		0.05 7	<0.0 01	0.00 1	8				<0.0 1		0.09 1	96	23	149				5.5	<0.0 05
WILPCUMBO_CO N	23-Dec- 2024	1130	2	0.21	<0.0 01	0.00 7	144		<1		541	<0.0 01		1		<1		0.99	<0.0 01		0.06 4	<0.0 01	0.00 1	7.9				<0.0 1		0.08 9	55	23	144				9.2	<0.0 05



#### **Surface Water Monitoring Locations**





LEGEND

Mining Lease Boundary
Mining Lease Application Boundary
Approved/Existing Open Cut and Contained
Infrastructure Area #
WCPL Monitoring

- ♦ WCPL Gauging Station
- ◆ EPL 12425 Licensed and Monitoring Point
- Active Surface Water Monitoring Site
- Historical Surface Water Monitoring Site

# Inclusive of the agreed minor change to the area confirmed by DPIE on 23rd August 2019.

Source: WCPL (2020); After DIPNR (2003); DPI Water (2015); NSW Spatial Services (2020)

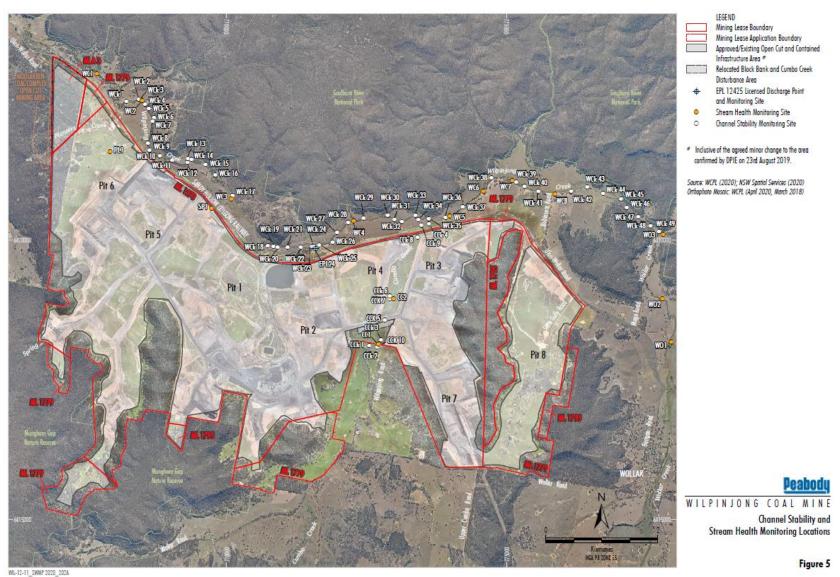


WILPINJONG COAL MINE

Wilpinjong Coal Mine Surface Water Monitoring Network

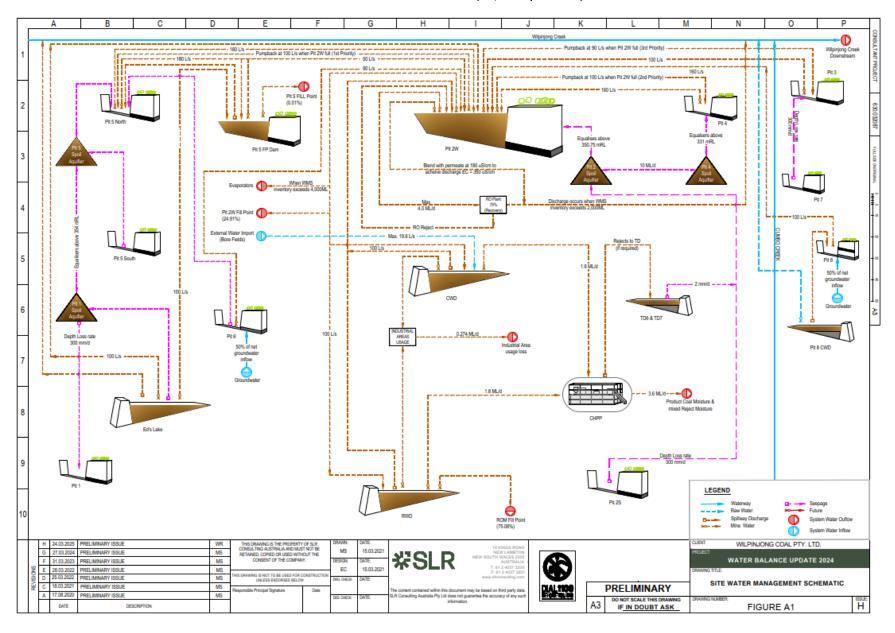


#### **Channel Stability & Stream Health Monitoring Locations**





#### Water Balance Model Schematic (SLR, SWB Update 2024)





# Water Management Performance Measures



A summary of the water management performance measures was undertaken by WCPL as they related to the Development Consent SSD-6764 (1 January 2024 to 31 December 2024)

#### Assessment of Water Management Performance Measures for 2024

Feature	Performance Measure	Complied with Performance Measure (Yes/No)	Comments/Actions
General	Maintain separation between clean, dirty and mine water management systems.  Minimise the use of clean water on site.  Design, install, operation and maintain water management systems in a proper and efficient manager.	Yes	Refer to Site Water Balance (Section 7.7) Refer to Estimate Groundwater Take (Section 7.2) Refer to Surface Water Results (Section 7.6)
Clean water diversion and storage infrastructure	Maximise as far as reasonable and feasible the diversion of clean water around disturbed areas on site.	Yes	Refer to Erosion and Sediment Control (Section 7.5)
Sediment dams	Design, install and/or maintain sediment dams to ensure no discharges to surface waters, except in accordance with an EPL or in accordance with Section 120 of the POEO Act.	Yes	Refer to Erosion and Sediment Control (Section 7.5) Refer to Water Treatment Facility (Section 7.8)
Mine water storages	Design, install and/or maintain mine water storage infrastructure to ensure no discharge of untreated mine water off-site.  Discharge treated mine water in accordance with an EPL or in accordance with Section 120 of the POEO Act.	Yes	Refer to Site Water Balance (Section 7.7) Refer to Surface Water Results (Section 7.6) Refer to Water Treatment Facility (Section 7.8)
Wilpinjong, Cumbo and Wollar Creeks	No greater impact than predicted for the development for water flow and quality.	Yes	Refer to Surface Water Results (Section 7.6) Refer to Stream Health (Section 7.9)
Aquatic, riparian and groundwater dependent ecosystems	Negligible environmental consequences beyond those predicted for the development.	Yes	Refer to Surface Water Results (Section 7.6) Refer to Stream Health (Section 7.9)
Flood mitigation measures*	Ensure all open cut pits, CHPP, coal stockpiles and main mine facilities areas exclude flows for all flood events up to and including the 1 in 100 year ARI.  All final voids designed to exclude all flood events up to include the PMF event.	Yes	The Wilpinjong Coal Mine open cuts are located outside the extent of flooding from Wilpinjong Creek in the 1 in 1,000 AEP design flood. Flood mitigation works for open cut infrastructure in the vicinity of Cumbo Creek are already being implemented at the Wilpinjong Coal Mine and have been designed to a 1 in 100 AEP flood protection (WRM Water and Environment, 2015).
Overburden, CHPP Reject and Tailings	Design, install and maintain emplacements to prevent or minimise the migration of pollutants due to seepage.	Yes	Waste rock emplacements and coal reject management in accordance with the MOP
Chemical and hydrocarbon storage	Chemical and hydrocarbon products to be stored in bunded areas or structures in accordance with relevant Australian Standards.	Yes	Chemical and hydrocarbon products stored in bunded areas in accordance with relevant Australian Standards (refer to IEA 2024)

**Notes:** \* Consistent with Condition 29, Schedule 3 of Development Consent (SSD-6764), WCPL have maintained all open cut pits, CHPP, coal stockpiles and main mine facilities areas so that they exclude flows for all flood events up to and including the 1 in 100 year ARI. The final voids would be designed to exclude all flood events up to the probable maximum flood.



# **Surface Water Reports 2024**







# Wilpinjong Coal Mine

**Annual Review 2024 – Surface Water Compliance** 

# Wilpinjong Coal Pty Ltd

1434 Ulan-Wollar Road WILPINJONG, NSW, 2850

Prepared by:

**SLR Consulting Australia** 

SLR Project No.: 665.v10014.02418

28 March 2025

Revision: 2.0

SLR Ref No.: 20250328\_SurfaceWaterAR\_2024-v2.0

#### **Revision Record**

Revision	Date	Prepared By	Checked By	Authorised By
1.0	26 March 2025	R Phayer and A Skorulis	N Bosworth	N Bosworth
2.0	28 March 2025	R Phayer and A Skorulis	N Bosworth	N Bosworth

# **Basis of Report**

This report has been prepared by SLR Consulting Australia (SLR) with all reasonable skill, care and diligence, and taking account of the timescale and resources allocated to it by agreement with Wilpinjong Coal Pty Ltd (the Client). Information reported herein is based on the interpretation of data collected, which has been accepted in good faith as being accurate and valid.

This report is for the exclusive use of the Client. No warranties or guarantees are expressed or should be inferred by any third parties. This report may not be relied upon by other parties without written consent from SLR.

SLR disclaims any responsibility to the Client and others in respect of any matters outside the agreed scope of the work.



## **Executive Summary**

This report documents the analysis and data considered for the review of flow and water quality trends at Wilpinjong Creek, Wollar Creek and Cumbo Creek near the Wilpinjong Coal Mine (WCM) to fulfil the surface water reporting requirements for the WCM 2024 Annual Review. The report is presented in three sections:

- 1. An overview of the volume and quality of discharge from the site under EPL 12425 including approved operational discharge from EPL Point 24 and EPL Point 30;
- 2. Analysis of flow and quality data from the Wilpinjong Creek and Cumbo Creek gauging stations, considering long-term rainfall trends, and licenced discharge from WCM.
- Assessment of electrical conductivity (EC), pH, and turbidity observations at Wilpinjong, Cumbo and Wollar Creeks during 2024 in respect to baseline data (pre-mining as defined in the Surface Water Management Plan (SWMP)) as well as Water Quality Impact Assessment Criteria for downstream monitoring sites within Cumbo and Wilpinjong Creeks, as defined in the current SWMP.

Discharge under EPL12425 from EPL Point 24 (the RO Plant) and EPL Point 30 (Pit 8 Clean Water Diversion (CWD) Dam) occurred within the stipulated discharge limits throughout 2023. It is noted that no discharge occurred from EPL Point 30 in 2024.

Analysis of continuous data at the WCM gauging stations in 2024 indicated generally higher flow conditions at Cumbo Creek and Wilpinjong Creek gauging sites compared to 2023 in response to above average rainfall in 2024 compared to below average rainfall conditions throughout 2023.

Within the reporting period, two Wilpinjong Creek downstream monitoring locations (WIL-D and WIL-D2) recorded exceedances of water quality monitoring criteria (pH upper limit). It is noted that the pH observations exceeding the upper trigger level for downstream Wilpinjong Creek may be within the normal range for pH at these locations. The 80th percentile pH from baseline data for these downstream sites is pH 7.9, which is above the established trigger level of pH 7.7.

A submitted but not yet approved version of the SWMP (from January 2025) has indicated that pH trigger levels at Wilpinjong Creek downstream monitoring sites should be updated to reflect downstream baseline data (pH 7.4 to 7.9) consistent with findings from a previous trigger exceedance investigation. This revised SWMP also indicates that pH trigger levels downstream of RO discharge at EPL Point 24 should reflect the EPL discharge limits when RO Plant discharge is higher than background flow in Wilpinjong Creek (as measured at WILGSU).

It is recommended that future assessments of pH consider these proposed revisions to the pH trigger levels to help evaluate whether trigger exceedances are likely to be adverse effects from Wilpinjong operations.



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# **Acronyms and Abbreviations**

ANZECC	Australian and New Zealand Environment and Conservation Council
ВОМ	Bureau of Meteorology
CCGSU	Cumbo Creek upstream
CRD	cumulative rainfall departure
CWD	Clean water diversion
EC	electrical conductivity
EMW	Excess Mine Water
EPL	Environmental Protection Licence
LDP	Licenced Discharge Points
MC	Moolarben Coal
mg/L	milligrams per litre
ML	megalitres
NTU	Nephelometric Turbidity Units
рН	pH unit
RO	Reverse Osmosis
SWMP	Surface Water Management Plan
TARP	Trigger Action Response Plan
TSS	Total Suspended Solids
μS/cm	Micro-Siemens per centimetre
WCM	Wilpinjong Coal Mine
WCPL	Wilpinjong Coal Pty Limited
WILGSD	Wilpinjong Creek gauging station downstream
WILGSU	Wilpinjong Creek gauging station upstream
·	



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### 1.0 Introduction

## 1.1 Background

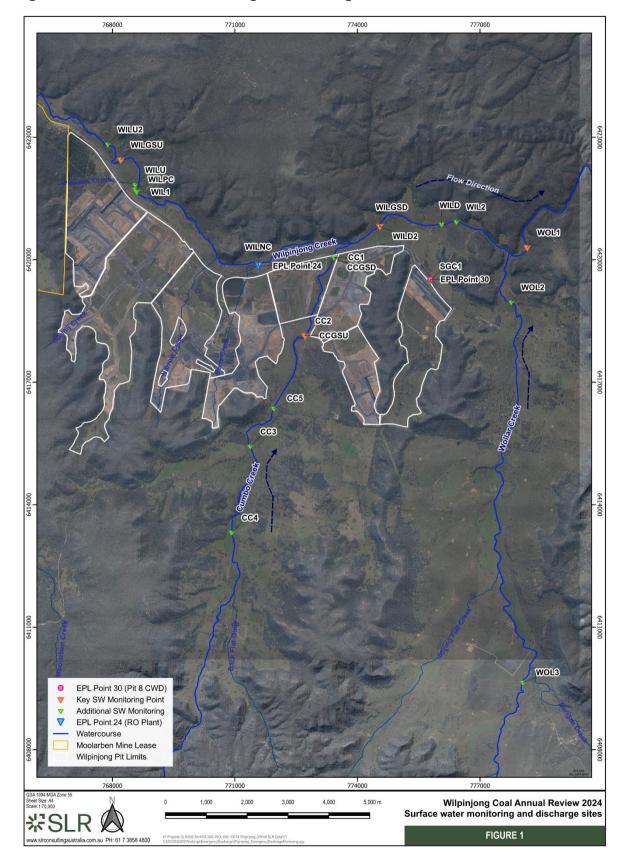
This report contains the analysis and information required for the 2024 Annual Review of flow and water quality trends at Wilpinjong Creek, Wollar Creek and Cumbo Creek near Wilpinjong Coal Mine (WCM). It serves as a supplementary document to the review of hydrogeological data conducted by SLR Consulting Pty Ltd (SLR) for the 2024 Groundwater Annual Review and 2023-24 Water Year Licensing Audit. This report presents information on the following items:

- 1 An overview of local climatic conditions experienced during 2024.
- 2 An overview of the volume and quality of water discharged from WCM during 2024 at the Licenced Discharge Points (LDPs) permitted under the Wilpinjong Coal Pty Limited (WCPL) Environmental Protection Licence (EPL) EPL12425.
- 3 Cause-and-effect analysis of data from the Wilpinjong Creek upstream (WILGSU) and downstream (WILGSD), and Cumbo Creek upstream (CCGSU) gauging stations, compared to the long-term rainfall trend and discharge from WCM and other regional mines.
- 4 Assessment of key water quality criteria at the local creeks during the 2023-2024 water year in respect to the baseline data (pre-mining, as defined in the Surface Water Management Plan (SWMP)), as well as Water Quality Impact Assessment Criteria for downstream monitoring sites within Cumbo and Wilpinjong Creeks, also defined in the current SWMP.

The Wilpinjong surface water quality monitoring, flow gauging stations and discharge locations are presented in **Figure 1**.



Figure 1: Surface water monitoring and discharge sites





### 2.0 Climate

**Table 1** displays the monthly and annual rainfall records from 2016-2024 compared to the long-term averages at the Wollar (Barrigan St) BOM station. The annual total rainfall recorded in 2024 was 779.4 mm, which equates to 131.41% of the long-term average of 593.1 mm, representing above average rainfall year.

**Table 2** presents the rainfall observed at the on-site rainfall gauge during 2024. Overall, rainfall recorded on-site at WCM is slightly lower than at the Wollar BOM station with a total for 2024 of 717.5 mm.

Variation in annual rainfall is a key influence on surface water flow and can influence water chemistry.

Table 1: BOM rainfall station 062032 - recent monthly and annual rainfall vs long term average (mm)

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Avg	67.2	62.2	55.1	39.3	37.0	43.7	42.9	41.1	41.7	52.1	57.0	60.9	593.1
2016	101.2	10.4	21.4	3.0	67.0	114.2	82.4	44.0	181.2	74.2	41.0	36.2	776.2
2017	13*	31.0	127.0	19.0	24.4	12.0	1.4	25.6	2.0	30.0	62.6	86.4	421.4
2018	13.4	66.2	41.4	47.0	12.6	22.0	6.5	25.5	51.0	48.5	44.4	117.6	496.1
2019	72.0	5.0	110.5	0.0	20.0	6.0	4.0	10.0	23.0	7.0	30.0	6.0	293.5
2020	37.0	151.0	110.2	118.0	35.0	31.3	86.0	36.0	75.7	128.0	21.5	149.3	979.0
2021	43.8	107.0	157.5	2.5	11.0	82.0	68.2	21.0	45.0	72.0	183.0	134.0	927.0
2022	169.0	17.0	139.5	65.0	38.0	14.5	109.0	100.5	94.5	126.0	85.0	31.0	989.0
2023	49.0	28.5	55.0	43.5	4.0	30.5	24.0	39.0	16.5	42.5	97.5	85.5	515.5
2024	88.5	39.5	71.0	73.9	63.5	81.5	60.0	42.5	36.0	52.0	103.0	68.0	779.4

<sup>\*</sup>No rainfall recorded at Wollar (Barrigan St). Rainfall from Bylong (Glenview) - 062107 used.

Table 2: Wilpinjong site rainfall data 2024

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
2024	78.6	32.8	75	61.8	69.2	58.8	39.8	42.5	36	52	103	68	717.5

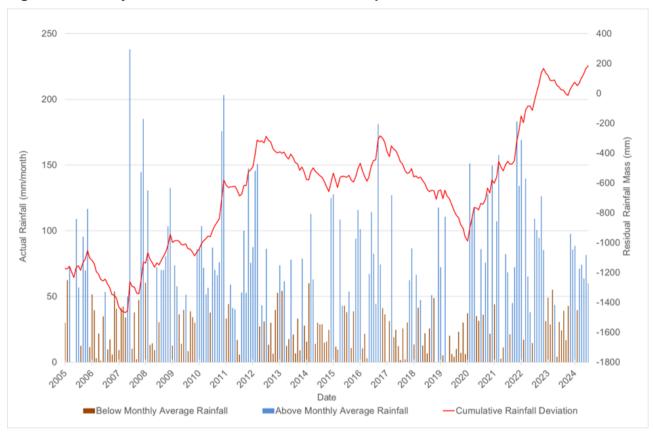
The cumulative rainfall departure (CRD) shows trends in actual rainfall over time relative to the long-term average and provides a historical record of relatively wet and dry periods. A positive slope in the CRD indicates periods of above average rainfall, while a negative slope indicates periods of below average rainfall. A level trace indicates rainfall conditions are equal to average rainfall conditions.

The CRD from the Wollar (Barrigan St) BOM station **Figure 2**, observes for the previous year 2023 a decline in the CRD, given the lower-than-average annual rainfall conditions. The last two months in 2023 see an increase in CRD consistent with higher rainfall in November and December. The upward trend in the CRD continued for 2024 reflecting the above-average rainfall conditions.



<sup>+</sup>Orange shading represents below average rainfall years whilst blue shading represents above average rainfall years.

Figure 2: Monthly rainfall and Cumulative Rainfall Departure





## 3.0 Discharge quantity and Quality

The following sections present a summary of the licenced discharge of water from WCM under emergency and normal EPL discharge provisions.

## 3.1 Emergency Discharge

There were no discharges under emergency provisions in 2024.

### 3.2 Licensed Discharge

Under EPL 12425, WCM is allowed to discharge water from site to Wilpinjong Creek from the following locations (see **Figure 1**):

- EPL Point 24 Product water from the RO treatment plant is discharged to Wilpinjong Creek. The daily discharge limit from the RO Plant is 6.5 ML/day. The EPL stipulates required monitoring of electrical conductivity (EC), pH, oil and grease, and total suspended solids (TSS).
- EPL Point 30 Discharge from the Pit 8 clean water diversion (CWD) dam to the
  downstream reach of Slate Gully Creek before it enters Wilpinjong Creek. There is no
  daily discharge limit and the EPL reflects Wilpinjong Coal's position that the water
  quality (i.e., measured as turbidity) from the Pit 8 CWD dam is generally equal to or
  better than the receiving water in Wilpinjong Creek.

The following sections provide further detail on the EPL conditions at these discharge points, and an overview of the quality and volume of water discharged in 2024. The quality of discharged water will contribute to water quality observations in Wilpinjong Creek and may be relevant when assessing surface water compliance for 2024.

#### 3.2.1 **EPL Point 24 – RO Plant**

WCM was historically approved to discharge up to 5 ML/day via the RO plant at EPL Point 24, which treats water from the on-site water retention dams. On 10 October 2022, EPL 12425 was updated to increase the discharge limit at EPL Point 24 to 6.5 ML/day. EPL 12425 specifies limits for the quality and monitoring frequency of water that may be discharged from this location (**Table 3**).

Table 3: EPL Point 24 - RO Plant Discharge Limits

Parameter	Unit of Measurement	Required Monitoring Frequency	Limit
EC	μS/cm	Continuous during discharge	500
Oil and Grease	mg/L	Weekly during any discharge	10.0
рН	pH unit	Continuous during discharge	6.5 - 8.5
TSS	mg/L	Weekly during any discharge	50
Discharge Vol. ML		Continuous during discharge	6.5

The recent recorded discharge volumes and associated water quality (EC and pH) from the RO plant are presented in **Figure 3**, which presents daily mean values for discharge from continuous monitoring alongside the weekly laboratory samples (RO Discharge Lab) for EC and pH. The monitoring and reporting against EPL Point 24 limits (**Table 3**) in 2024 are summarised in **Table 4**.



Table 4: EPL Point 24 2024 Monitoring

EPL Reporting	Comment					
Freq.	Monitoring Freq	Water Quality Limits				
12x monthly reports completed in 2024	Parameters requiring continuous monitoring were collected 84% of the time in January 2024 and then 100% of the time from February to December 2024.	EPL Point 24 limits are not exceeded for any analytes during 2024.				
	Data provided to SLR for this review indicates that Oil and Grease and TSS samples were collected at the required frequency throughout 2024.					

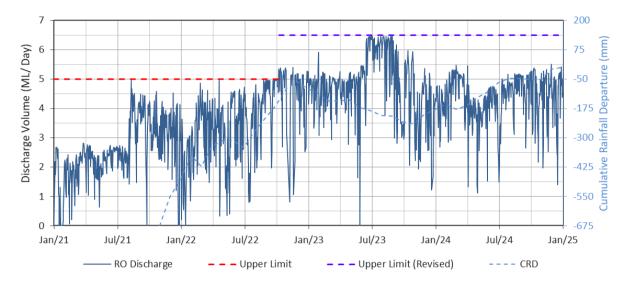
#### 3.2.2 EPL Point 30 – Pit 8 CWD dam

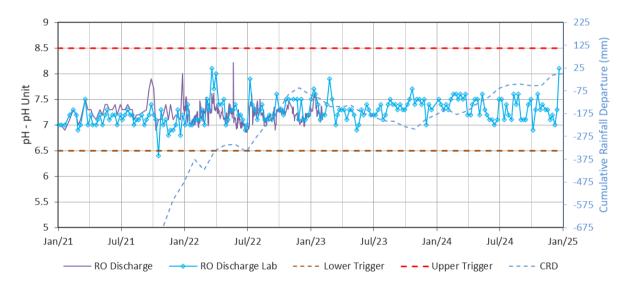
WCM discharges surface water run-off captured above mining operations at EPL Point 30. This area above the mining operations is referred to as the Pit 8 clean water diversion (CWD) dam. The turbidity value measured in the discharge at EPL Point 30 should not exceed the turbidity value measured at the Wilpinjong Creek upstream gauging station (WILGSU). The water discharged from EPL Point 30 is captured rainwater and should therefore have a water quality (i.e. turbidity) that is equal to or better than the turbidity of the receiving water in Wilpinjong Creek. When there is no flow within Wilpinjong Creek at the upstream gauging station the value of turbidity measured at EPL Point 30 must not exceed 50 Nephelometric Turbidity Units (NTU), which is a 'limit' recommended in the 'Blue Book' (Soils and Construction Volume 1 – Managing Urban Stormwater – Landcom, 2004).

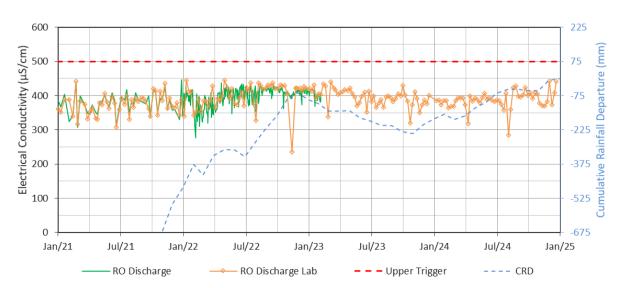
No discharge from EPL Point 30 occurred in 2024.



Figure 3: RO Plant discharge volume and quality in 2024









## 4.0 In-Stream Monitoring Data Review

Flow rates and water quality (pH and EC) are monitored continuously from two sites on Wilpinjong Creek (WILGSU and WILGSD) and one site on Cumbo Creek (CCGSU).

The locations of the gauging stations on Wilpinjong Creek are shown in **Figure 1**. The upstream site (WILGSU) is located northwest of WCM. The downstream site (WILGSD) is northeast of WCM, downstream of the RO Plant and downstream of the confluence of the Wilpinjong and Cumbo creeks. The Cumbo Creek upstream gauging station (CCGSU) is located approximately 400 m to the east of Pit 2 and approximately 800m upstream of Pit 4 which is now used for water storage and rehabilitation (**Figure 1**). Flow/discharge, EC, and pH are all monitored at these locations.

Real-time flow and water quality data was provided up to 10 December 2024.

#### 4.1 Surface Water Flow

The following section presents and discusses daily flow data from the three continuous surface water monitoring gauges on Wilpinjong Creek (WILGSU and WILGSD) and Cumbo Creek (CCGSU). Observed flow trends are reviewed against rainfall data from the local rainfall station (Wollar, 062032) and discharge volumes throughout 2024.

The two Wilpinjong Creek gauging stations have been recording since January 2012. The catchment area reporting to the upstream site (WILGSU) is 86 km2 while the downstream site has a catchment area of 216 km2. CCGSU on Cumbo Creek has been recording data since August 2015. **Figure 4** shows the flow data at these sites from late 2022 to the end of 2024 in comparison to the RO Plant discharge rate (EPL Point 24).

During 2024, flow at CCGSU fluctuated between <0.01 and 7.6 ML/day in response to rainfall events with an average flow of 0.3 ML/day. CCGSU was observed to flow for most of the year except for three brief periods in May, June, August and December.

In 2024, flow at WILGSU ranged between <0.01 and 15 ML/day with an average of 0.3 ML/day, while WILGSD had recorded flows from 0.2 to 14.5 ML/day at an average of 5.0 ML/day. Flow rates at WILGSD directly influenced by RO Plant discharge volumes (**Section 3.2.1**)

**Table 5** presents the calculated daily mean discharge rates at WILGSU, WILGSD and CCGSU for each year since 2013. The average daily flow rate of all creek monitoring points increased from 2019 through 2022 with all sites showing a reduction in daily averages for the 2023 and 2024 reporting periods.

Table 5: Calculated daily mean flow rate at Wilpinjong and Cumbo Creeks

Location	rivolage bany i len itale (inbrady)											
ID	2013	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024
WILGSU	0.16	0.03	0.24	2.8	0.002	0	0	5.2	5.1	25.8	1.1	0.3
WILGSD	0.27	0.22	0.39	5.7	5.9	0.73	0.008	6.0	10.0	70.0	6.3	5.0
CCGSU	N	lo data	0.14	1.6	0.6	0.4	0.1	0.9	2.1	20.4	0.95	0.3



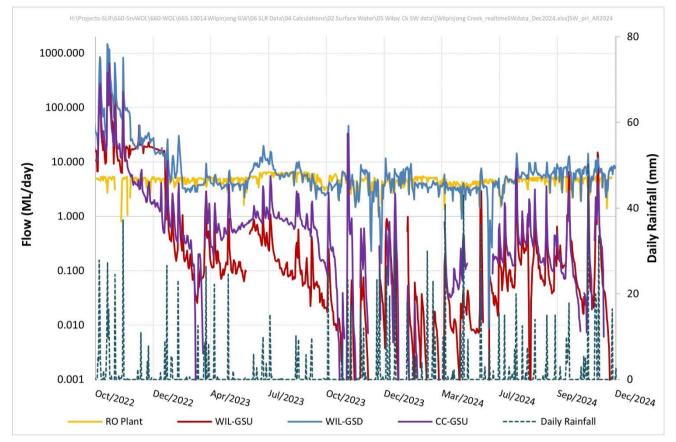


Figure 4: Continuous flow monitoring records (Oct 22 – Dec 24)

## 4.2 Water Quality

Water quality is monitored continuously at WILGSU, WILGSD and CCGSU, with a multi parameter water meter (sonde) measuring EC, pH (and temperature, which is not provided or assessed here).

### 4.2.1 Electrical Conductivity

EC monitoring data at WILGSU, WILGSD and CCGSU are provided in **Figure 5** and are generally influenced by the following factors:

- WILGSU is most strongly influenced by the rainfall trend, with limited contribution identified from groundwater (baseflow). EC at WILGSU is therefore generally low (~1,000 to 2,000 μS/cm) and relatively consistent, with a minor inverse response to the rainfall trend (lower rainfall results in an increase in EC) likely resulting from increased evaporation and lower contribution of fresh water in periods of low rainfall.
- Flow at WILGSD is influenced by upstream flow from both Wilpinjong and Cumbo Creeks as well as the RO Plant discharge, which all have different EC values. EC at WILGSD is therefore variable and related to the primary source of flow at any point in time.
- Flow at CCSGU is likely to have a persistent groundwater contribution that is sourced from weathered Permian Coal Measures. In 2024, observations of EC are in the range between 3,000 μS/cm and 5,000 μS/cm but historically have been observed



higher than  $8,000~\mu\text{S/cm}$ . Declines in EC are observed following large rainfall events, due to dilution effects.

In 2024 continuous monitoring at Cumbo Creek (CCGSU) shows generally stable EC from March to September ( $\sim$ 3,000-4,000  $\mu$ S/cm) before increasing EC to  $\sim$ 5,000  $\mu$ S/cm later in 2024. EC trends in 2024 are consistent with responses observed historically in CCGSU.

Both WILGSU and WILGSD displayed generally stable EC levels across 2024 of around 750  $\mu$ S/cm upstream and 1,000  $\mu$ S/cm downstream, with short-term fluctuations linked to changing flow conditions and inputs.

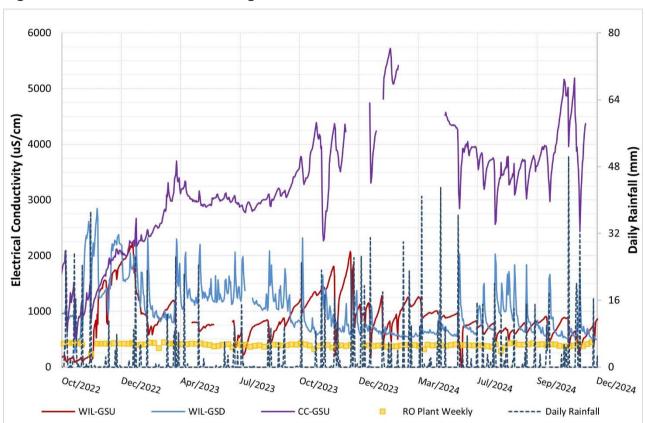


Figure 5: Continuous EC monitoring at WCM

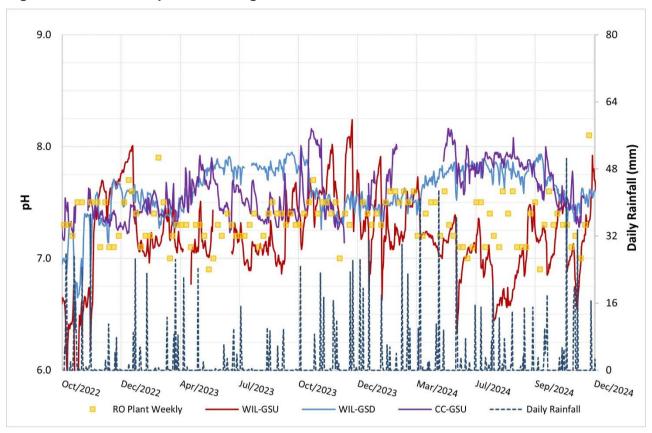
#### 4.2.2 pH

pH at CCGSU is generally consistent throughout 2024, with pH around pH 7.25 to 8.0, showing minor decreases following periods of higher rainfall, which has lower pH (**Figure 6**).

pH at both gauging stations on Wilpinjong Creek are different by about 1 pH unit and show some correlation to periods of rainfall (declining with higher rainfall periods). For most of 2024 the pH levels in Wilpinjong Creek show some variability that appears linked with periods of high rainfall. WILGSD varies from pH 7.2 to 8.0 and WILGSU varies from pH 6.3 to 8.1.



Figure 6: Continuous pH monitoring





# 5.0 Water Quality Analysis

The following sections review surface water quality data from monitoring sites specified in Section 8 of the Surface Water Management Plan (WCPL, 2017). This review has been conducted with respect to the 20th and 80th percentile values from baseline monitoring data (**Table 6**) (which was collected from 2004 to 2009, prior to the commencement of mining) and water quality impact assessment criteria (trigger levels), where defined (**Table 7**).

Section 5.1 provides an assessment of the 2024 water quality observations with respect to the impact assessment criteria, while **Sections** 5.2 to 5.6 comment on the 2024 observations with respect to the baseline water quality data.

Table 6: Summary of Baseline Water Quality Data - Local Creeks (WCPL, 2017)

Monitoring Site/Gu	рН	EC (µS/cm)	Turbidity	
Monitoring Sitered	ilucilile	βII	LC (μο/ciii)	(NTU)
ANZECC (2000) Guideline Trigger Value	Protection of Aquatic Ecosystems	6.5-8.0	30-350	2-25
	Primary Industries (Livestock Drinking Water)	6-9	950	-
Wilpinjong Creek Upstream (Sites	Average	7	2,435	20
WIL-U2, WIL-U, WIL 1, WIL-PC)	Minimum	5.7	450	6
, -,	Maximum	9	12,190	41
	No. Samples	49	49	5
	80 <sup>th</sup> percentile	7.7	4,066	24
	20 <sup>th</sup> percentile	6.9	-	
Wilpinjong Creek Downstream	Average	8	3,531	22
(Sites WIL-NC, WIL-D2, WIL 2, WIL-D)	Minimum	6.7	680	4
,	Maximum	9	7,450	70
	No. Samples	55	55	9
	80 <sup>th</sup> percentile	7.9	5,166	28
	20 <sup>th</sup> percentile	7.4	-	-
Cumbo Creek Upstream (Sites	Average	8	5,303	11
CC2, CC3, CC4, CC5)	Minimum	6.8	100	5
	Maximum	9	10,500	24
	No. Samples	70	70	15
	80 <sup>th</sup> percentile	8.2	6,750	16
	20 <sup>th</sup> percentile	7.4	-	-
Cumbo Creek Downstream (Site	Average	8	6,231	43
CC1)	Minimum	6.7	540	17
	Maximum	9	10,470	94
	No. Samples	27	27	6



Monitoring Site/Gเ	рН	EC (µS/cm)	Turbidity (NTU)		
	80 <sup>th</sup> percentile	8.2	7,510	77	
	20 <sup>th</sup> percentile	7.52	-		
Wollar Creek (Sites WOL 1, WOL	Average	8	2,311	16	
2, WOL 3)	Minimum	6.5	90	2	
	Maximum	8.4	6,540	37	
	No. Samples	90	90	20	
	80 <sup>th</sup> percentile	8.0	3,460	25	
	20 <sup>th</sup> percentile	7.4	-	-	

Where trigger levels are defined (**Table 7**) the review will identify any exceedances during 2024 and provide preliminary analysis.

Table 7: Water Quality Impact Assessment Criteria (WCPL, 2017)

Creek	Monitoring Site	Parameter	Trigger				
Wilpinjong Creek	WIL_NC, WIL_D2, WIL_D, WIL_2	EC	If recorded value at the monitoring site is greater than <b>3,440 µS/cm</b> for 3 consecutive readings				
(Downstream)		Turbidity	If recorded value at the monitoring site is greater than <b>24 NTU</b> for 3 consecutive readings				
		pH (lower)	If recorded value at the monitoring site is less than <b>6.9 pH</b> for 3 consecutive readings				
		pH (upper)	If recorded value at the monitoring site is greater than <b>7.7 pH</b> for 3 consecutive readings				
Cumbo Creek (Downstream)	CC1	EC	If recorded value at the monitoring site is greater than <b>7,510 µS/cm</b> for 3 consecutive readings				
		Turbidity	If recorded value at the monitoring site is greater than <b>77 NTU</b> for 3 consecutive readings				
		pH (lower)	If recorded value at the monitoring site is less than <b>7.5 pH</b> for 3 consecutive readings				
		pH (upper)	If recorded value at the monitoring site is greater than <b>8.2 pH</b> for 3 consecutive readings				

<sup>&</sup>lt;sup>1</sup> Trigger is only considered to have been exceeded if the recorded value at a monitoring site is greater than (or less than for lower pH Trigger) all values from the upstream monitoring sites sampled on the same day. In the event that a single result is recorded above/below the 80th/20th percentile value, WCPL will undertake a preliminary investigation to ascertain whether the result was caused by an obvious anomaly or whether further testing is required.



# 5.1 Assessment with respect to SWMP (WCPL, 2017) water quality triggers

**Table 8** identifies Water Quality Impact Assessment Criteria defined in the SWMP (WCPL, 2017) that have been exceeded during 2024. This assessment, in line with the SWMP (WCPL, 2017) has only considered triggers to be exceeded under the following circumstances:

- Trigger is only considered to be exceeded if recorded value at the monitoring site is greater than (or less than for lower pH trigger) for 3 consecutive readings.
- Trigger is only considered to have been exceeded if the recorded value at monitoring site is greater than (or less than for lower pH Trigger) all values from the upstream monitoring sites sampled on the same day.

Table 8: Exceedances of Water Quality Impact Assessment Criteria (WCPL, 2017)

Creek	Site	Parameter	Trigger	Exceedance during 2024	Summary of Exceedance
Wilpinjong	WIL-	EC	3,440 µS/cm	No	
Creek (Downstream)	NC, WIL-D2,	Turbidity	24 NTU	No	
, ,	WIL-D, WIL-2	pH (lower)	6.9 pH (7.4 pH proposed)	No	
		pH (upper)	7.7 pH (pH 7.9 proposed)	Yes	All observations from May to December 2024 are above the upper pH trigger, and above upstream pH observations at WIL-D2 during 2024.  Observations are above the upper pH trigger at WIL-D from April to September and late October to December 2024.  See Section 5.3.1.1and Figure 7.
Cumbo Creek	CC1,	EC	7,510 µS/cm	No	
(Downstream)	CC-1- 30m-up,	Turbidity	77 NTU	No	
	CC-GS- D	pH (lower)	7.5 pH	No	
		pH (upper)	8.2 pH	No	

During the 2024 assessment period, the identified trigger exceedances are upper pH trigger exceedances at Wilpiniong Creek (Downstream), as observed at WIL-D2 and WIL-D.

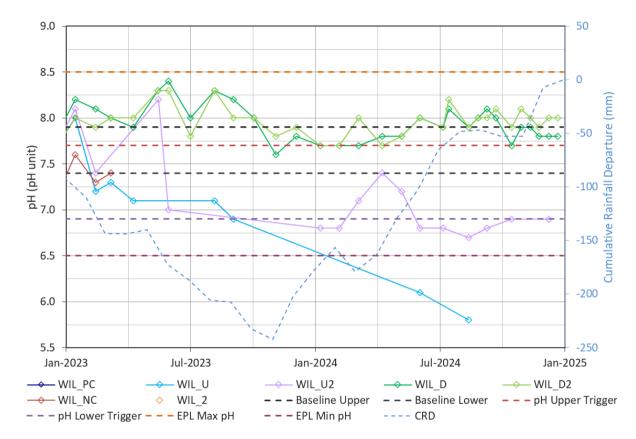
A review of possible and plausible drivers of this trigger exceedance is provided in **Section 5.3.1.1** while **Figure 7** shows time-series pH observations at all Wilpinjong Creek compliance monitoring sites to visualise the frequency and magnitude of 2024 trigger exceedances.

Baseline pH from Wilpinjong Creek downstream monitoring sites as well as pH limits discharge from EPL Point 24 (the RO Plant) are included in **Figure 7**. A submitted but not yet approved version of the SWMP (from January 2025) has indicated that pH trigger levels



at Wilpinjong Creek downstream monitoring sites should be updated to reflect downstream baseline data (pH 7.4 to 7.9 – See Table 8 of the approved SWMP Peabody (2017)). This revised SWMP also indicates that pH trigger levels downstream of RO discharge at EPL Point 24 should reflect the EPL discharge limits when RO Plant discharge is higher than background flow in Wilpinjong Creek (as measured at WILGSU).

Figure 7: Wilpinjong Creek pH at compliance monitoring locations





## 5.2 Wilpinjong Creek Upstream

The reach defined as Wilpinjong Upstream (WCPL, 2017) is assessed using monitoring data from sites WIL-U2, WIL-U, WILGSU and WIL-PC **Table 6**). These sites are located along Wilpinjong Creek near the northwestern edge of the current and proposed WCM mining activity (**Figure 1**). **Figure 8** presents time series pH, EC and turbidity data for these monitoring locations.

### 5.2.1 pH

pH observations at the Wilpinjong Creek Upstream monitoring sites during 2024 are relatively stable and near neutral, with pH generally ranging from pH 6.7 to 7.6. Two lower pH observations occurred at WIL-U (pH 5.8 and pH 6.1) in the two months (June and August 2024) where flow was observed, and a sample was taken. No flow was observed at WIL-U in other 2024 observations and the observed pH is within the limit of previous Wilpinjong Creek Upstream observations.

Rainfall, and subsequent flow conditions are considered to be the primary drivers of fluctuations in the pH observations at upstream Wilpinjong Creek monitoring sites.

### 5.2.2 Electrical Conductivity

EC observations at Wilpinjong Creek Upstream monitoring sites have shown considerable variation between 2006 and 2024 (<1,000  $\mu$ S/cm to 6,000  $\mu$ S/cm). EC is more elevated in historical observations (>4,000  $\mu$ S/cm) at WIL-U, WIL-U2, and WIL-PC, and are observed to occur simultaneously with fresher observations at WIL-GS-U (~2,000  $\mu$ S/cm). More saline observations earlier in the monitoring record may indicate some component of groundwater flow from the underlying Permian coal measures. Mining of the Ulan Seam within these coal measures at WCM and adjacent Moolarben will depressurise the coal measures to some extent and may reduce the more saline contribution to Wilpinjong Creek.

A notable freshening at all Wilpinjong Creek Upstream sites occurs in late 2020 to the end of 2022 (generally <750  $\mu$ S/cm), in response to above average rainfall conditions. An increase in EC (to ~1,000  $\mu$ S/cm) is observed during the below average (2023) and near average (2024) rainfall and lower flow period of 2023 and 2024 (Section 4.1). EC observations at Wilpinjong Creek Upstream monitoring sites are well below the 80th percentile baseline (4,066  $\mu$ S/cm) for all of 2024.

### 5.2.3 Turbidity

Turbidity observations at Wilpinjong Creek Upstream monitoring sites continuously fluctuate between 2010 and 2024, with observations ranging from 5-2,000 NTU and are above the 80th percentile baseline monitoring value (24 NTU) for around half of the observations. Turbidity observations with higher values generally appear to be associated with periods of below average rainfall, particularly periods after low and no-flow conditions.

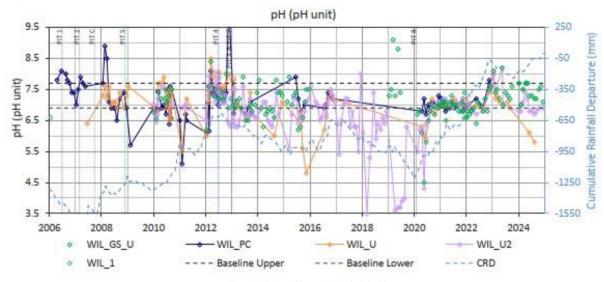
During 2024, turbidity observations ranged from 6.6 – 174 NTU with an average of 42.5 NTU. This is well within the range of historical observations but is above the baseline turbidity value of 24 NTU. Future investigation could be considered if turbidity remains high throughout 2025.

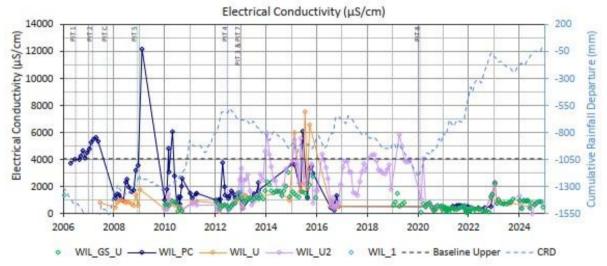
Flow conditions (influenced by rainfall trends) are considered to be the primary drivers of turbidity observations at Upstream Wilpinjong Creek monitoring sites. If no influence of operations from WCM or upstream Moolarben can be attributed to turbidity observations consistently reporting above the 80th percentile baseline value, future reviews, or updates of

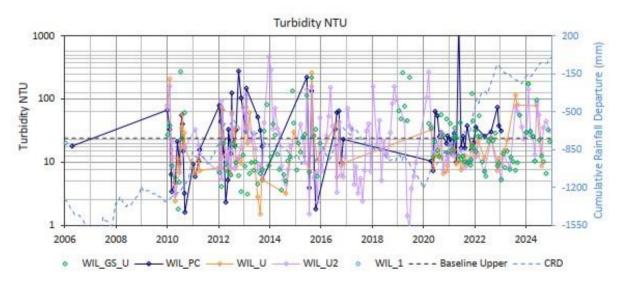


the SWMP could consider updating the baseline period to better capture fluctuations under normal conditions.

Figure 8: Time-series water quality for Wilpinjong Creek Upstream









## 5.3 Wilpinjong Creek Downstream

The creek reach defined as Wilpinjong Creek Downstream (WCPL, 2017) is assessed against water quality trigger levels at sites WIL-NC, WIL-D2, WIL-D and WIL-GS-D (**Table 7**). These sites are located along Wilpinjong Creek, adjacent to, or just downstream of the WCM mining operations and confluence with Cumbo Creek (for sites other than WIL-NC) (**Figure 1**). The full series of available data is shown in **Figure 9**.

### 5.3.1 pH

During 2024, pH observations at Wilpinjong Creek Downstream monitoring sites are above the upper trigger level (pH 7.7) for the majority of measurements at WIL-D and WIL-D2. pH observed at Wilpinjong Creek Upstream monitoring sites (WIL-U and WIL-U2) are lower than downstream observations (WIL-D and WIL-D2) for all of 2024, therefore breaching the upper pH trigger level as defined in the SWMP (WCPL, 2017) (Table 7).

WIL-NC was not monitored in 2024 although flow was noted at this location for October-December observations. Monitoring at WIL-D and WIL-D2 increased in frequency (twice per month) from July to December 2024 to collect additional data associated with the trigger exceedance. Observations at WIL-D and WIL-D2 ranged from pH 7.7 to pH 8.2 with an average of pH 7.9 in 2024.

Time series pH observations for 2024 at all compliance monitoring locations on Wilpinjong Creek are presented in **Figure 7**, above.

These pH exceedances at Wilpinjong Creek Downstream monitoring sites were documented and assessed by SLR (2023c) consistent with requirements of the surface water Trigger Action Response Plan (TARP) (Peabody, 2017 – Table 15).

### 5.3.1.1 Trigger Exceedance

The following points provide an evaluation of the pH trigger exceedance and consider whether it is likely related to WCM operations. These points draw on the detailed trigger investigation undertaken in SLR (2023c):

- Higher pH surface water is naturally occurring in the Wilpinjong area (Cumbo Creek and Wollar Creek). pH at downstream Wilpinjong Creek is also generally higher than upstream. Historical observations from 2007 and 2013/14 are also above pH 8.
- Discrepancy was identified between real-time datasets (WIL-GSD, Figure 6) and manual (monthly or rain event) pH observations (Figure 7). Variations in these datasets increase uncertainty when evaluating data and identifying potential mining effects.
- Cumbo Creek is likely influencing the water signature of Wilpinjong Creek (downstream of the confluence) and is likely contributing bicarbonate alkalinity to Wilpinjong Creek. Higher bicarbonate alkalinity is linked to higher pH at Wilpinjong (SLR, 2023c).
- There is no clear evidence of elevated levels in site water storages which could result in the seepage of mine water to Wilpinjong Creek. However, this possibility should remain a consideration.
- Higher pH surface water is present locally, outside the influence of WCM operations.
  The exceedance is therefore unlikely to pose a threat to the health of local
  ecosystems. Observations in 2023 of pH 8.6 at upstream Wollar Creek (WOL2), and
  ~pH 8.5 at upstream Cumbo Creek (CC-3) are consistent with available historical
  data.



As specified in the last surface water annual review and SLR Trigger Exceedance assessment (2023c), baseline pH data collected for downstream Wilpinjong Creek sites have a 20th percentile value of pH 7.4 and an 80th percentile value of pH 7.9 (**Table 6**). Therefore, under normal conditions, pH observations are expected to be higher than pH 7.9, around 20% of the time, meaning a trigger level of pH 7.7 may be too low to meaningfully indicate a potential Wilpinjong Coal mining effect that justifies further investigation.

A submitted but not yet approved version of the SWMP (from January 2025) has indicated that pH trigger levels at Wilpinjong Creek downstream monitoring sites should be updated to reflect downstream baseline data (pH 7.4 to 7.9) based on the SLR (2023c) investigation. This revised SWMP also indicates that pH trigger levels downstream of RO discharge at EPL Point 24 should reflect the EPL discharge limits for pH (pH 6.5 - 8.5) when RO Plant discharge is higher than background flow in Wilpinjong Creek (as measured at WILGSU).

Under these proposed updates, the pH trigger level would not have been exceeded in 2024, noting that RO discharge was consistently higher than flow from Wilpinjong Creek upstream.

### 5.3.2 Electrical Conductivity

As discussed in **Section 4.2**, EC observations at Wilpinjong Creek Downstream monitoring sites are influenced by upstream flow from Wilpinjong Creek, flow from Cumbo Creek, discharge permissible under EPL 12425, and some contribution of groundwater baseflow. This has resulted in higher EC observations in periods of low flow, which is attributed to greater contributions from baseflow or Cumbo Creek flow. Also observed are longer periods of consistently low EC observations from 2016 to 2018 attributed to low EC RO Plant discharge.

In 2024, EC observations are generally between 500-2,000 µS/cm and all observations are below the trigger level.

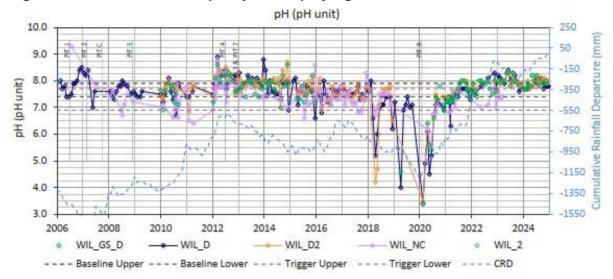
### 5.3.3 Turbidity

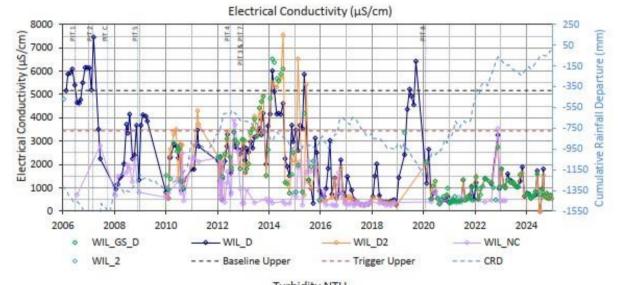
Turbidity observations at monitoring sites in the Wilpinjong Creek downstream sites show some variability from 2010 to 2024 (1-1,000 NTU) (**Figure 9**), with a minor inverse relationship to the rainfall trend.

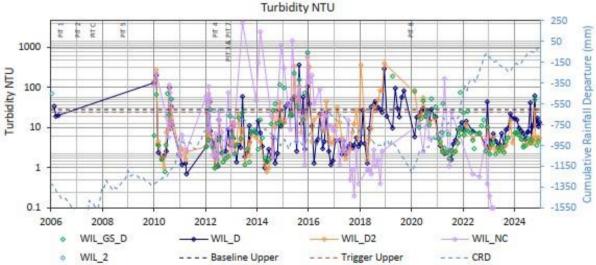
During 2024, turbidity observations at Wilpinjong Creek Downstream monitoring sites are generally below the 80th percentile baseline (28 NTU) and trigger level (24 NTU) and there are no consecutive observations above the trigger level.



Figure 9: Time-series water quality for Wilpinjong Creek Downstream









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### 5.4 Cumbo Creek Upstream

The creek reach defined as Cumbo Creek Upstream (WCPL, 2017) is assessed using monitoring data from sites CC2, CC3, CC-GS and CC-GS-U (**Table 6**). These sites are located along Cumbo Creek to the south of WCM (**Figure 1**). **Figure 10** presents time series pH, EC and turbidity data for these monitoring locations.

### 5.4.1 pH

pH observations at Cumbo Creek Upstream have been relatively stable from 2015 through 2024. The farthest upstream site, CC-3, has reported observations of around pH 8.3 over 2015-2024 while CC-2 and CC-GS-U were closer to pH 8. In 2024, pH observations at CC-2 and CC-GS-U were recorded within the 20th and 80th percentile baseline value whilst CC-3 showed more alkaline readings; this is consistent with observations since 2015 and earlier historical observations.

### 5.4.2 Electrical Conductivity

EC observations at Cumbo Creek Upstream show considerable variation for the entire period of monitoring (<1,000  $\mu$ S/cm to ~10,000  $\mu$ S/cm) but are generally brackish to saline. Freshening may occur following increases in the long-term rainfall trend as is seen in late 2016, and again from mid-2021 to the end of 2022, with the inverse observed in periods of low rainfall.

During 2024 EC observations continued to show an increasing EC trend that began in 2023. This is likely driven by a return to below average (2023) and near average (2024) rainfall conditions which may allow a greater proportion of groundwater discharge to Cumbo Creek compared to 2022 observations in high rainfall conditions. 2024 observations at all sites remain below the 80th percentile baseline (6.750 µS/cm).

A combination of rainfall, subsequent flow and ongoing baseflow contributions are considered to be the primary drivers of EC observations at Cumbo Creek monitoring sites.

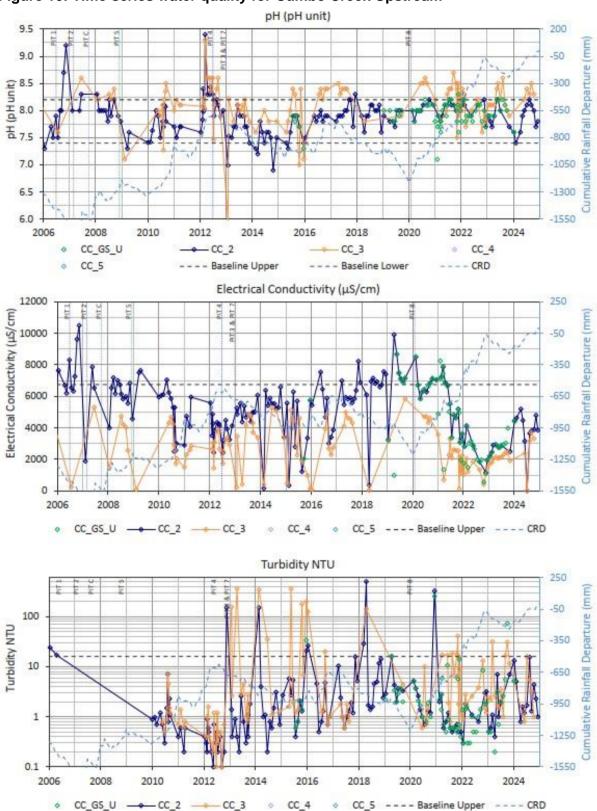
### 5.4.3 Turbidity

All turbidity observations at Cumbo Creek Upstream monitoring sites in 2024 are below the 80th percentile baseline value for data collected from 2004 to 2009 (16 NTU) with a maximum of 15.8 NTU observed at CC-3 in July 2024.



CC\_GS\_U

Figure 10: Time-series water quality for Cumbo Creek Upstream





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### 5.5 Cumbo Creek Downstream

The reach defined as Cumbo Creek Downstream is assessed against water quality trigger levels at site CC1, CC-GS-D, and CC-1-(up 30 m) (**Table 7**). These sites are located close to the confluence of the Wilpinjong and Cumbo Creeks and are near the northern extent of the WCM mining operations (**Figure 1**). No samples were taken at the alternate downstream Cumbo Creek site, CC-1-(up 30 m) during 2024. Access can be unsafe to this site, and sampling is frequently unsuccessful due to a lack of observable surface flow. It is therefore, not considered further in this analysis.

### 5.5.1 pH

From 2015 to early 2019, pH observations at Cumbo Creek Downstream monitoring sites were consistently below the trigger level defined in the SWMP (WCPL, 2017) at a level of around pH 7 (Figure 11). They were also generally lower than pH observations from Cumbo Creek Upstream monitoring sites (Figure 10).

Throughout 2024 both monitoring sites, CC-1, and CC-GS-D, were within the pH trigger levels (pH 7.5-8.2) at the Cumbo Creek downstream sites.

### 5.5.2 Electrical Conductivity

EC observations at Cumbo Creek Downstream monitoring sites show considerable variation from 2015 through 2024 (<1,000  $\mu$ S/cm to ~6,400  $\mu$ S/cm) but have not recorded an observation above the trigger level since 2015 (7.510  $\mu$ S/cm).

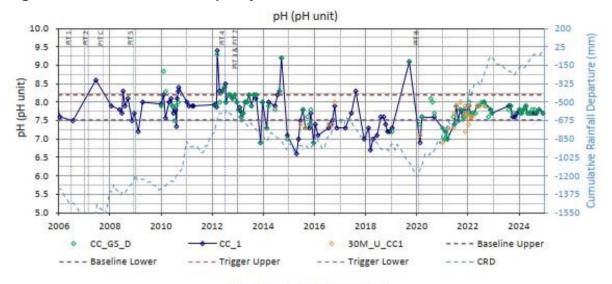
During 2024, EC observations at Cumbo Creek Downstream monitoring sites are well below the trigger level (7,510 µS/cm) with readings around 4,000 µS/cm.

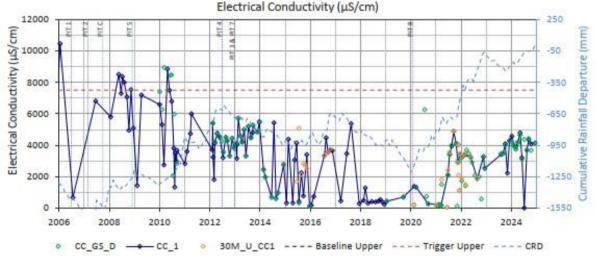
### 5.5.3 Turbidity

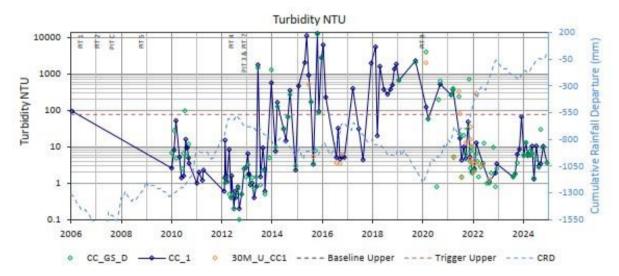
All turbidity observations at Cumbo Creek Downstream monitoring sites in 2024 were below the trigger level (77 NTU).



Figure 11: Time-series water quality for Cumbo Creek Downstream









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### 5.6 Wollar Creek

Wollar Creek is assessed using monitoring data from sites WOL1 and WOL2 (Figure 1). The sites are located along Wollar Creek to the east and south of WCM, with WOL1 located downstream of the confluence between Wilpinjong and Wollar Creeks. The Wollar Creek monitoring sites are located approximately 5 km from the current extent of the WCM mining activity.

### 5.6.1 pH

pH observations at Wollar Creek have been relatively stable from 2015 through 2024. WOL-1 and WOL-2 observations have been marginally higher than the 80th percentile value (pH 8.0), with a range of pH 7.8 to 8.6 in WOL-1 and pH 8.0 to 8.4 seen in WOL-2.

The observations at both sites are generally consistent with observations from previous years though WOL-2 has shown a general increase in pH over time, with 2024 values higher than generally observed in historical data at approximately pH 8.0.

### 5.6.2 Electrical Conductivity

EC observations at both Wollar Creek monitoring locations show some influence from rainfall as well as baseflow from more saline groundwater.

In 2024, all EC observations are below the 80th percentile baseline values (<3,500 µS/cm).

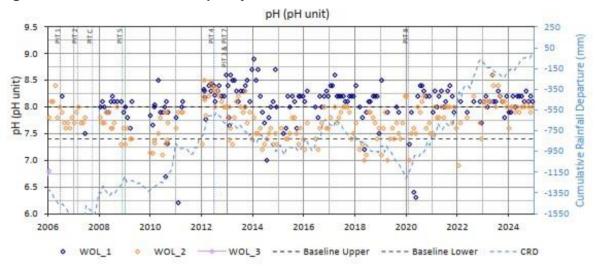
### 5.6.3 Turbidity

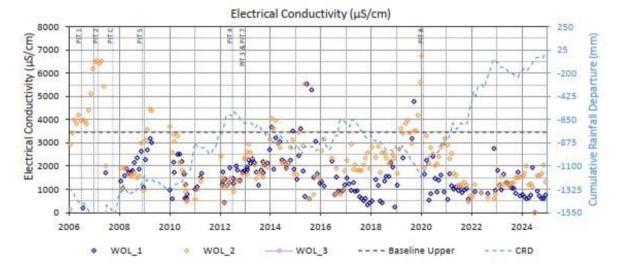
Turbidity observations at Wollar Creek monitoring sites have been relatively stable from 2015 through 2024 and have generally been recorded below the 80th percentile of baseline data collected from 2004-2009 (25 NTU).

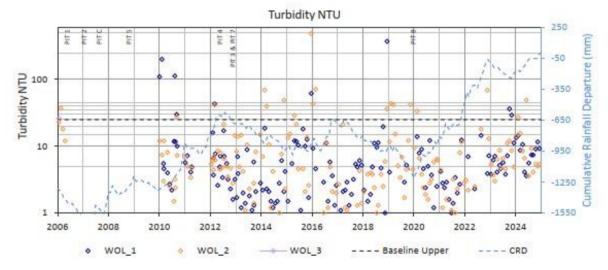
Turbidity observations during 2024 at Wollar Creek monitoring sites were below the 80th percentile baseline (25 NTU) aside from two observations at WOL-2 (February and June 2024). Overall, turbidity readings for 2024 are consistent with observations for the entire monitoring period (2006-2024).



Figure 12: Time-series water quality for Wollar Creek









### 6.0 Recommendations

The only TARP exceedances experienced in 2024 are that associated with pH observations above the trigger level in the Wilpinjong Creek Downstream monitoring locations.

SLR completed the preliminary investigation of the pH trigger exceedances within the WCM surface water monitoring network (SLR, 2023c) at Wilpinjong Creek downstream sites, consistent with the trigger action response plan (TARP) for surface water quality (SWMP Table 15 – WCPL, 2017).

The submitted but not yet approved version of the SWMP (from January 2025) has indicated that pH trigger levels at Wilpinjong Creek downstream monitoring sites should be updated to reflect downstream baseline data (pH 7.4 to 7.9) consistent with findings from SLR (2023c). This revised SWMP also indicates that pH trigger levels downstream of RO discharge at EPL Point 24 should reflect the EPL discharge limits when RO Plant discharge is higher than background flow in Wilpinjong Creek (as measured at WILGSU).

It is recommended that future assessments of pH consider these proposed revisions to the pH trigger levels to help evaluate whether trigger exceedances are likely to be adverse effects from Wilpinjong operations.



### 7.0 References

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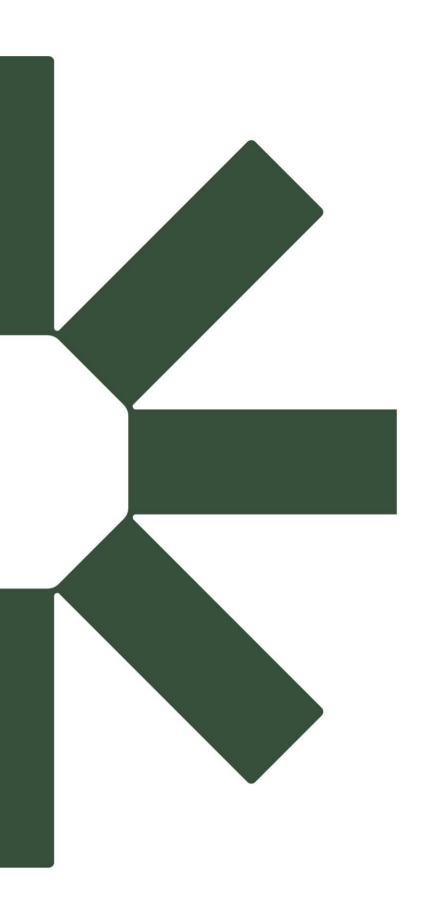
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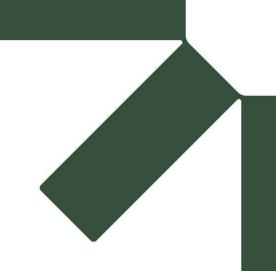
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# **Site Water Balance Model**

# **Model Update and Calibration 2025**

# **Wilpinjong Coal Mine**

# Wilpinjong Coal Pty Ltd

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SLR Project No.: 630.032087.00001

27 March 2025

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### **Revision Record**

Revision	Date	Prepared By	Checked By	Authorised By
v1.0	27 March 2025	Walter Rowlands	Paul Delaney	Paul Delaney
v0.1	25 March 2025	Walter Rowlands	Paul Delaney	Paul Delaney

# **Basis of Report**

This report has been prepared by SLR Consulting Australia (SLR) with all reasonable skill, care and diligence, and taking account of the timescale and resources allocated to it by agreement with Wilpinjong Coal Pty Ltd (the Client). Information reported herein is based on the interpretation of data collected, which has been accepted in good faith as being accurate and valid.

This report is for the exclusive use of the Client. No warranties or guarantees are expressed or should be inferred by any third parties. This report may not be relied upon by other parties without written consent from SLR.

SLR disclaims any responsibility to the Client and others in respect of any matters outside the agreed scope of the work.



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# **Appendices**

Appendix A Model Schematic	Appendix A	A Model	<b>Schematic</b>
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Appendix B **Catchments and Land Use** 

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### 1.0 Introduction

Wilpinjong Coal Pty Ltd (WCPL) operates the Wilpinjong Coal Mine (WCM) which is located approximately 40 km north-east of Mudgee in the Mid-Western region of New South Wales (NSW).

WCPL have developed and continue to maintain a water balance simulation model for the WCM. Initially, the site utilised OPSIM simulation software calibrated to monitoring data between January 2014 and January 2018. The model was then redeveloped in 2020 by SLR Consulting Pty Ltd (SLR, 2020a) using the GoldSim software package and data obtained between January 2018 and December 2019. The GoldSim model has been recalibrated annually since its inception in 2020, with the most recent calibration prior to this study completed in March 2024 as part of the previous annual review (SLR, 2024a).

WCPL are required to prepare a site water balance in accordance with Condition 30(d)(ii), Schedule 3 of Development Consent SSD-6764. WCPL have engaged SLR to review and update the WCPL Water Balance Model (WBM) to capture changes to the site water catchments and management system during 2024 and calibrate the WBM using monitoring data collected up to the end of December 2024.

This report documents the model update process and outcomes, including:

- Collation and review of historical water monitoring data;
- Review of WCPL's harvestable rights for 2024;
- Updated catchment and land use mapping and changes incorporated to the Water Management System (WMS) in 2024;
- Calibration of WCPL's GoldSim model against the 2024 GoldSim output and data collected between January 2018 and December 2024;
- Description of the GoldSim model, operating rules, and model schematic; and
- Forecast of site water behaviour for the next three years (2025 to 2027).

The intent of this report is to document the basis of the updated WCPL GoldSim model, assess the predicted water balance versus actual monitored water inventory during 2024, and to provide a 3-year forward projection of water balance at WCM.



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# 2.0 Background

### 2.1 Operational Description

The WCM is an open cut thermal coal mine located approximately 40 km north-east of Mudgee near the village of Wollar, within the Mid-Western Regional Local Government Area (LGA) in central NSW.

WCM is owned and operated by WCPL, a wholly owned subsidiary of Peabody Energy Australia Pty Ltd (Peabody). The WCM ("the mine" or "the site") extracts Run-of-Mine (ROM) coal from the Ulan Seam or Moolarben Coal Member which is either processed on site at the coal handling and preparation plant (CHPP) or bypassed directly to product stockpiles. Current approvals permit production of up to 16 million tonnes per annum (Mtpa) of ROM coal. Coal products are transported by rail on the existing Sandy Hollow Gulgong Railway to domestic energy generators and to the Port of Newcastle for export (Resource Strategies, 2015).

The WCM has eight approved open cut mining areas, named Pit 1 through to Pit 8. Mining is currently undertaken in Pits 1 to 8. Open cut mining of Pit 1, 2 and 5 historically originated at a point and has progressed outward, forming a series of peripheral excavations separated by backfilled spoil. These sub-pits are defined based on their relative position within the associated main pit, i.e., Pit 5 South (Pit 5S), Pit 5 North (Pit 5N) and so on (WRM, 2019).

WCM is located adjacent to the right (southern) bank of Wilpinjong Creek, which is incised into a valley between the sandstone plateaus of the Munghorn Gap Nature Reserve to the south, and the Goulburn River National Park to the north. The mine is located on the alluvial/colluvial flats associated with the gullies draining the southern escarpment. The valley flats have typical gradients toward Wilpinjong Creek of approximately 1 in 65 (1.5%). The escarpment rises approximately 100 m from the valley floor to elevations exceeding 450 m Australian Height Datum (mAHD) on the plateau. The sandstone plateaus are heavily forested. The surrounding valley flats are used for cattle and sheep grazing with intermittent cropping, principally for fodder (WRM, 2015).

A general arrangement plan of WCM as of 31 December 2024 is provided in Figure 1.

### 2.2 Approvals and Licences

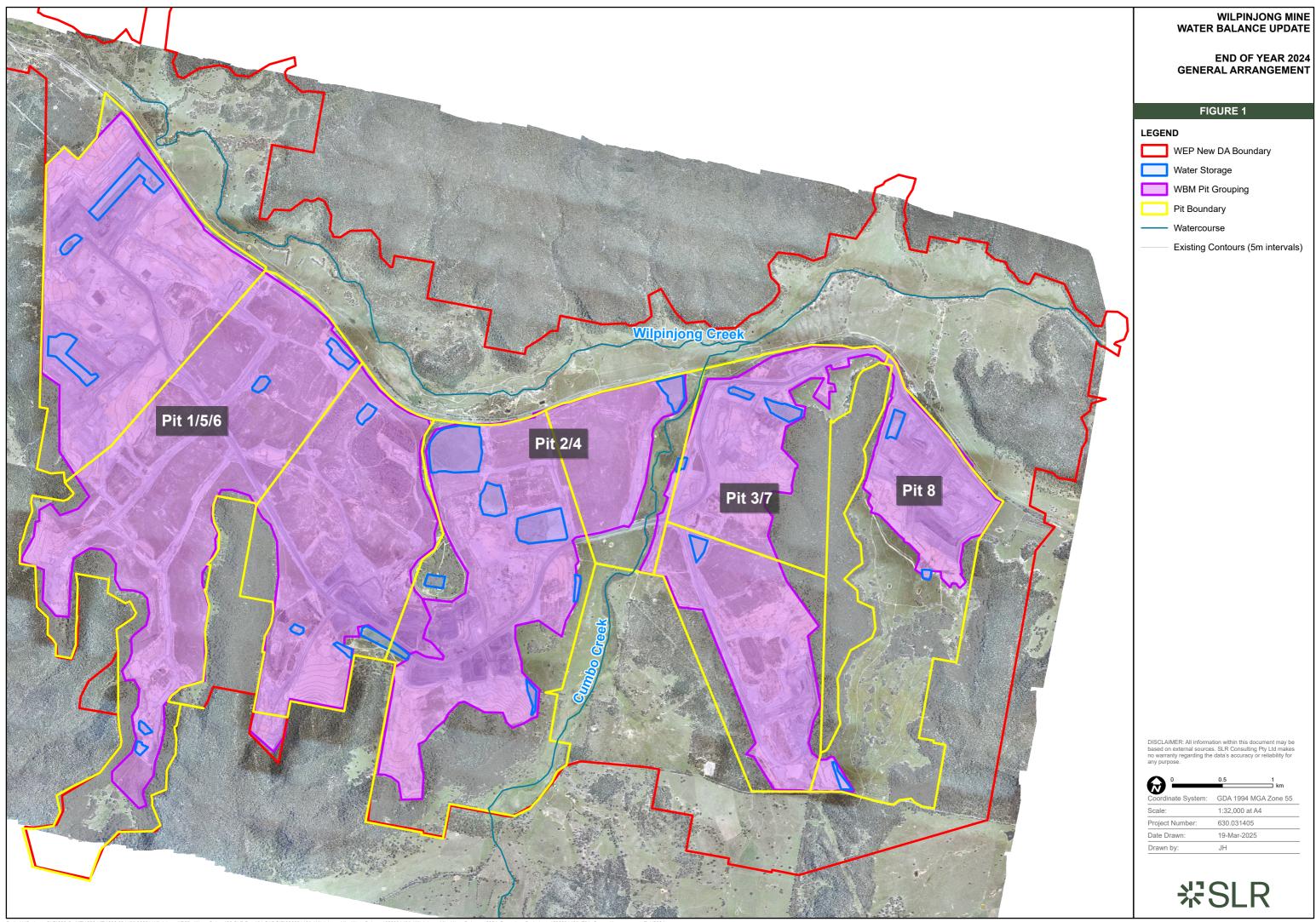
WCM originally operated under Project Approval 05-0021 that was granted by the NSW Minister for Planning under Part 3A of the NSW Environmental Planning and Assessment Act 1979 (EP&A Act) on 1 February 2006.

On 24 April 2017, WCPL was granted Development Consent SSD 6764 for the Wilpinjong Extension Project (WEP) that provides for the continued operation of WCM at rates of up to 16 Mtpa of ROM coal out to 2033, and access to approximately 800 hectares (ha) of open cut extensions. Development Consent SSD 6764 has superseded the Project Approval 05 0021, which was surrendered on 28 April 2020 as required under SSD-6764.

WCM is also subject to conditions outlined in Environmental Protection Licence (EPL) No. 12425.

Mining operations are carried out upon Mining Leases (MLs) 1573, 1779, 1795 and 1846 in accordance with the Rehabilitation Management Plan (RMP), a requirement of MLs and SSD-6764.





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# 3.0 Water Management System

### 3.1 Overview

The WCM Water Management System (WMS) comprises a network of internal dams interconnected via pumps/pipelines and drainage channels. The main objective of the WMS during wet periods is to minimise the risk of uncontrolled discharge of water to the receiving environment and to minimise the risk of pit inundation which may impact coal production. During dry periods, the main objective of the WMS is to ensure that adequate reserves are available to maintain water supply for mining operations. If required, WCM have access to a water supply bore field which can be activated to import external water during these periods. The majority of the system's water storage capacity is provided by Pit 2W, a former open cut mining pit located adjacent to Ulan-Wollar Road. Other significant water storages include the Recycled Water Dam (RWD) and Clean Water Dam (CWD) (refer **Figure 1**).

WCM currently has eight open cut mining pits (Pit 1, 2, 3, 4, 5, 6, 7 and 8). Review of deepest mined topographic data shows that historical mining has occurred within three distinct voids, which each share a common and continuous pit floor, and are divided from each other by an unmined in-situ rock barrier. These voids are referred to herein as Pit 1/5/6 (containing Pits 6, 5S, 5N, and 1), Pit 2/4 (containing Pits 2W, 2S, 2E and 4) and Pit 3/7 (containing Pits 3 and 7). Pit 1/5/6 and Pit 2/4 feature a central overburden emplacement area, which acts as a highly permeable aquifer. During 2024, mining activities proceeded in Pits 5, 6, 7 and 8.

Water within each void passively drains to the north down the dip of the former coal seam, collecting in either Pit 3, Pit 4, Pit 5N, or Pit 8 where it is then pumped to the Pit 2W hub water storage. Note that the Pit 1/5/6, Pit 2/4 and Pit 3/7 definitions are only used in the context of water management; these definitions do not align with mine planning terminology.

Water inflows to the WMS include rainfall, catchment runoff and groundwater interception. The mine has intersected several ephemeral creeks, and these catchments now report to the WMS. It is also noted that WCM's mine rehabilitation is still progressing in accordance with the RMP and those completed rehabilitated areas have not yet had sufficient time to mature to the extent that would allow runoff from these areas to be discharged off-site in compliance with approval conditions.

Water is used for dust suppression (road watering, stockpile sprays), wash down (wash bays and vehicle wash stations) and for coal washing. The majority of water used for these applications is lost via evaporation or entrainment within railed product coal and waste rock dumps. The coal washing process formerly included a wet-tailings circuit, with tailings slurry pumped to a number of approved Tailings Dams (TDs) adjacent to Pit 2W for consolidation and water recovery (note that tailings was pumped into two approved TDs located at the northern end of Pit 1 prior to using the Pit 2 TDs).

The process was modified in April 2015 to include a tailings belt filter press (BFP). Mixed reject is now co-disposed of within the overburden dumps. TD1 to TD5 have been capped and rehabilitated. TD6 remains active to allow for the deposition of tailings slurry during periods in which the BFP is undergoing maintenance. As TD6 is projected to reach capacity in 2025-26 if the tailings deposition rate determined in 2023 is maintained at the site (SLR, 2023), WCPL are currently progressing the design of TD7. This structure is proposed immediately northwest of TD6 within the former Pit 2 mine void, at a downgradient location that currently receives seepage from the northwest corner of TD6.

During periods of heightened water inventory, WCM operates a Water Treatment Facility (WTF) which utilises reverse osmosis (RO) technology and discharges a blend of permeate



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and Pit 2W water to the adjacent Wilpinjong Creek in accordance with flow and water quality limits specified in EPL 12425.

Prior to 2018, the WTF comprised a WCPL owned primary plant, supplemented with a second leased plant installed to provide temporary additional treatment/discharge capacity. The temporary WTF was decommissioned at the beginning of 2018. WTF reject was pumped to Pit 1S and/or the RWD until late 2018 when Pit 1S was taken offline and was mined through in early 2019. WTF reject, along with backwash from the WTF and water that doesn't meet the requirements outlined in EPL 12425, is now directed to Pit 2W and/or the RWD.

During periods of low water inventory associated with extended drought, WCM are licenced to draw water from a network of water supply bores to supplement site water demands. WCM also import potable water which is used to supply amenities. In 2025, the site plans to supplement potable water at the mining camp with treated water from Pit 2 via the water treatment facility. Sewage is either treated and disposed on site via irrigation in accordance with EPL 12425 or occasionally removed from site by a licenced contractor to be processed through a licenced facility. The potable water circuit has no functional influence on the performance of the WMS and is not discussed further in this study.

The following subsections summarise the physical characteristics of the WCM water management system, including water storage specifications and function, catchment and land use classification breakdown, and key transfer infrastructure specifications as incorporated in the model.

### 3.2 Water Storage infrastructure and Voids

### 3.2.1 Function and Specifications

**Table 1** summarises the location, specifications and description for key water storages and voids within the WMS. Consistent with documentation associated with previous water model updates, infrastructure has been grouped as follows:

- Water Storages: Infrastructure used for storing water that has come into contact with mining operations. Comprises surface ponds/dams and inactive mining pits used for bulk water storage;
- **Sediment Dams:** Sumps/dams used to intercept and capture sediment laden runoff generated from disturbed areas. Water captured in these structures is pumped back to the mine WMS:
- Tailings: Dams or repurposed open cut mining pits used to store tailings waste. Note
  that tailings storage capacities have not been listed in the following tabulation, as
  available space in these structures is not intentionally used for water storage; or
- **Mining Pits:** Open cut voids currently subject to active mining. Intercept surface water and groundwater prior to transfer to the WMS. Not used for water storage (unless required to prevent off-site discharge to the environment).

Table 1: Key Water Storage and Void Specifications with Functional Descriptions

Storage		ation Zone 55)	Catchment	Full Storage Capacity		Functional Description
	Easting	Northing	(ha)	(mAHD)	(ML)	
Water Storages						



Storage	Location (GDA94 Zone 55)		Catchment	Full Storage Capacity		Functional Description
g-	Easting	Northing	(ha)	(mAHD)	(ML)	,
Pit 2 West (Pit 2W)	770975	6419350	415.1	398	4,088	Hub water storage, and primary buffer storage. Receives dewatering from mining and processing areas and supplies water to industrial tasks as required. Feed water supply for the WTF.
Pit 1 South (offline from late 2018)	769250	6417120	-	421.4*	295*	Stores reject from the WTF.
Pit 5 Fill Point (FP) Dam	769030	6419995	33.2	392.2	8	Water supply for dust suppression activities in the Pit 5 mining area. Water makeup from local mining area dewatering, or Pit 2W as a backup.
Clean Water Dam (CWD)	770785	6418000	2.1	397	51	Water supply for CHPP/ Mine Infrastructure Area (MIA). area tasks. Water makeup from Pit 2W.
Recycled Water Dam (RWD)	770270	6417430	26.7	412.6	295	Water supply for CHPP/MIA area tasks and to the ROM truck fill point. Water makeup from Pit 2W. May also receive concentrate from the WTF.
Ed's Lake	770085	6419690	292.4	375.3	110	Transfer dam located in backfilled Pit 1N void. Storage capacity includes basin to the north-east of the main void storage. Also captures water from the Pit 1 rehab area.
MIA Dam	770570	6417820	-	-	-	Sediment trap located near admin area. Intercepts sediments from water draining back to Pit 2W from the CHPP/MIA.  Note: not included in GoldSim model.
Pit 8 CWD	775683	6418277	306.8	-	25	Captures majority of Pit 8 upslope catchment via Pit 8 upstream diversion. Constructed March 2020. Two downstream farm dams capture overflow which in turn overflow to Pit 8. Discharge of clean water from the Pit 8 CWD is via LDP 30.
Sediment D	ams					
Pit 5N Sed. Dams	769530	6420700	-	-	-	Sediment interception works located adjacent to open cut workings.
Pit 2E Sed. Dams	772800	6418580	-	-	-	Function is to capture sediment laden runoff, allowing this water to then be
Pit 3 Sed. Dams	773850	6420010	-	-	-	pumped back to the WMS.  Note: these dams have been functionally
Pit 7 Sed. Dams	773240	6417880	-	-	-	modelled as additional catchment assigned to their respective open cut void (i.e. assumes no storage in sediment ponds,
Pit 8 Sed. Dams	775782	6419484	-	-	-	and no pumping constraints).
Mining Pits						
Pit 5 South	767730	6418020	592.7	n/a	n/a	
Pit 5 North	769220	6420690	732.3	n/a	n/a	
Pit 1	769440	6417660	297.7	n/a	n/a	Active mining nite
Pit 2 South	771250	6416940	37.9	n/a	n/a	Active mining pits.
Pit 2 East	772070	6417900	33.1	n/a	n/a	
Pit 4	772840	6419850	132.5	n/a	n/a	



Storage	Storage Location (GDA94 Zone 55) Catchme (ha)		Catchment	Full Storage Capacity		Functional Description		
			(IIa)	(mAHD)	(ML)			
Pit 3	773840	6419230	294.0	n/a	n/a			
Pit 7	774210	6417780	300.8	n/a	n/a			
Pit 6	767950	6420330	411.8	n/a	n/a			
Pit 8	775851	6419225	149.5	n/a	n/a			
Tailings Sto	Tailings Storage							
TD6	771800	6418530		n/a	n/a	TD6 is an active tailings dam used		
TD7	771320	6418860	78.7	n/a	n/a	intermittently when the BFP is offline. The TD7 structure is currently under design and yet to be commissioned. At present, the proposed TD7 footprint receives rainfall-runoff contributions and seepage from the northwest corner of TD6.		

Note:

### 3.2.2 Storage Characteristics

Storage characteristics (stage-storage relationships) remain consistent with the previous model update (SLR, 2024a).

Modelled stage-storage profiles for all storages have been provided for reference in **Appendix D.** 

### 3.2.3 Storage Capacities

### 3.2.3.1 Water Storages

Adopted Full Storage Levels (FSLs) for all water storages are listed in Table 2.

Table 2: Adopted Full Storage Levels for Site Water Storages (Source: WRM, 2019)

Storage	FSL (mAHD)	Basis		
		As per the stage storage provided by WCPL from recent Bathymetri survey (July 2023).		
Pit 1 South (offline from late 2018)	422	Nominal 0.5 m offset below the level at which additional seepage flows to Ed's Lake were inferred as part of the WBM verification (WRM, 2019).		
Pit 5 Fill Point (FP) Dam	392	Defined based on review of 2019 surface topography. Nominal level at which overflow to Pit 5N would occur.		
Clean Water Dam	397	Maximum water level recorded in historical water level survey. FSL defined as a maximum operating level rather than a spillway level. Comprises an agricultural concrete overflow point.		
Dirty Water Dam	413	It is understood that this dam seeps to the CHPP area at high water levels, and water levels in the dam are managed to minimise the risk of this occurring. FSL defined as an operational level rather than a spillway level. It is understood that this dam has no formally constructed spillway outlet (WRM, 2019).		
Ed's Lake	375	Defined based on review of 2019 surface topography. Nominal elevation at which overflow to Wilpinjong Creek would occur via a low point in adjacent road/rail.		
Pit 8 CWD	-	Dam has a capacity of 25 ML.		



<sup>\*2018</sup> data prior to decommissioning.

### 3.2.3.2 Open Cut Pits

To prevent an uncontrolled release of water to the receiving environment, excess mine water would be temporarily stored within one or more open cut mining pits. This practice would continue until the excess water is drawn down through evaporation, supplied to demands (e.g., dust suppression) or via EPL Licensed Discharge Point (LDP) LDP 24 (via the site's WTF).

The assumed order of preference in which pits would be filled is Pit 3, Pit 4 then Pit 5N. Note that water storage in up-dip pits (i.e., Pit 5S, Pit 1, Pit 2S, Pit 7, Pit 6) is not possible as these voids freely drain down the dip of the coal seam, through the in-pit spoil placement areas to their respective down-dip pits.

Overflow and recommended maximum fill levels are listed in **Table 3**. Recommended maximum fill levels reflect settings incorporated into the WBM for current storage capacities. Recommended fill levels have been set five metres below the nominal overflow level. Actual fill levels (which trigger filling of the next pit in sequence) should continue to be confirmed/defined to reflect changes due to mine progression.

Table 3: Mining Pit Overflow and Recommended Maximum Fill Levels

Pit	Level (	(mAHD)	Notes				
FIL	Overflow	Max Fill					
Pit 5N	381.0	369.0	Assumed hydraulic connection between Pit 5N and Ed's Lake.				
Pit 4	366.0	362.0	Overflow level based on low point in northern end of Pit 4N high wall.				
Pit 3	362.0	358.0	Overflow level based on low point on western side of Pit 3N void (adjacent to Cumbo Creek).				

#### 3.2.4 Catchment Breakdown

Catchment boundaries for water storages within the WCM have been delineated based on the most recent available topographic data and advice from operational personnel. Catchment areas for 2024 have been summarised in **Table 1**. Catchment and land use maps are provided in **Appendix B**.

Land use classifications used for the model calibration have been determined based on review of end of year 2024 satellite imagery.

Current investigations have adopted a land use classification schedule to align with catchment yield parameters:

- **Natural / undisturbed** no disturbance, typically grass or brush;
- Roads / industrial / hardstand / mining Pit sealed or unsealed road or track, cleared and compacted earth or concrete (layout areas etc.), open-cut void;
- **Spoil / overburden** unrehabilitated spoil emplacement, clear of vegetation, also includes cleared areas and beach and other exposed tailings reject areas; and
- Rehabilitated overburden emplacement areas that have been shaped and revegetated.

Land use data has been used to calculate catchment yield within the WBM. Different land use classifications generally correspond with a unique catchment runoff model parameter set. Catchment yield is discussed further in **Section 4.4**.

A breakdown of land use type per water storage catchment area has been provided in **Appendix B**, in addition to catchment and land use plans.



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#### 3.2.5 Water Transfer Infrastructure

The WCM transfer network comprises a mixture of fixed pump and pipeline infrastructure connections, supplemented with portable infrastructure that can be moved around for pit dewatering. Water transfer capacities adopted as part of the WCM WBM are consistent with the previous model update and are summarised in **Table 4**. Active management of Pit 8 commenced in 2020 and pumped discharge from the Pit 8 CWD via EPL Licensed Discharge Point LDP 30 commenced in 2021 and are included below.

The following assumptions towards water transfer infrastructure have been applied:

- Assumed no pumping from up-dip pits, i.e., Pit 5S, Pit 1, Pit 2S, Pit 2E and Pit 7.
  These pits passively drain along the dip of the mined coal seam (either along the surface or through the highly permeable in-pit spoil placement areas) to their respective down-dip pits;
- Water transfers from dams for industrial tasks are assumed to be constrained by demand, not by pump/pipeline capacity; and
- Assumed no pumping from any tailings dams water inflow to these areas is assumed to evaporate or seep to the underlying Pit 2/4 spoil aquifer which is hydraulically connected to Pit 2W.

The WBM includes a time delay for transfer of water volumes by seepage, and this parameter has been progressively calibrated within annual WBM updates.

**Table 4: Water Transfer Infrastructure Modelled Capacities** 

Catagony	Conne	ction Points	Flow Capacity			
Category	Storage (From)	Directed (To)	L/s	ML/day		
	Pit 5N	Pit 2W	180 <sup>1</sup>	15.5		
	Pit 4	Pit 2W	160 <sup>1</sup>	13.8		
Pit Dewatering	Pit 3	Pit 2W	90	7.8		
	Pit 6	Pit 2W	100 <sup>3</sup>	8.6		
	Pit 8	Pit 2W	100	8.6		
	Ed's Lake	Pit 5 FP Dam	100	8.6		
Mine Water Containment	Ed's Lake	Pit 2W	100	8.6		
wine water Containment	Pit 2W	Pit 5N	100	8.6		
	Pit 2W	Pit 3N	100	8.6		
Controlled Discharge	Pit 8 CWD	Off-site <sup>2</sup>	200	17.3		
Other	Pit 2W	CWD	100	8.6		
Other	Pit 2W	RWD	100	8.6		

<sup>&</sup>lt;sup>1</sup> Dewatering capacity for active pits is variable, subject to allocation of pump resources.



<sup>&</sup>lt;sup>2</sup> Prior to 2021 water was pumped from Pit 8 CWD to Pit 2W at 160 L/s.

<sup>&</sup>lt;sup>3</sup> Dewatering of Pit 6 from 2023.

### 4.0 Climate

#### 4.1 Overview

Climatic influences on the WMS include catchment rainfall—runoff and evaporation (from wetted areas) and evapotranspiration (from catchments). The WBM has been configured to simulate system performance on the basis of long-term historical climate data. Historical data has been directly applied, based on the assumption that climatic conditions observed in the past, and captured in the data, are indicative of persistent local climatic trends. Historical data is therefore assumed to represent the range of potential conditions likely to be observed in the near future.

This investigation, and those prior, have not included allowance for climate change effects as these are unlikely to be material in the three-year forecasting period.

Updated climatic data for WCM (latitude -32.35, longitude 149.9) has been sourced from the SILO Data Drill service (Jeffrey et al., 2001). The Data Drill service accesses grids of climate data interpolated from point observations by the Bureau of Meteorology (BoM) for any point in Australia. Sourced information includes daily resolution rainfall and evaporation data, for the 125-year period of 1900 to present. This information has been processed and summarised in the following sub-sections.

WCPL have also provided rainfall data for the January 2016 to December 2024 period, recorded at the site automated weather station (AWS), located within the rail loop (near the CWD). Rainfall data recorded at the neighbouring BoM rainfall gauge at Wollar (Wollar Barrigan St Station 062032) has also been sourced and used for reference. Site AWS and BoM rainfall data have been compared against Data Drill rainfall in **Section 4.2**.

### 4.2 Rainfall

### 4.2.1 Annual Rainfall (Data Drill)

Annual rainfall at the site varies between approximately 200 mm and 1,200 mm ( $\sim$ 1,000 mm range), with a median of 608 mm  $\pm$  183 mm. Approximately 70% of the dataset falls within 1 standard deviation of the median. Annual rainfall totals (calendar year) have been presented in on a percentile basis in **Figure 2**.

WCM experienced drought conditions across 2017 to 2019. During this period, the 2017 and 2018 rainfalls were equivalent to historical 21st and 26th percentile (dry), respectively, whilst the 2019 rainfall was equivalent to a historical 1st percentile (very dry). In contrast, rainfall experienced during 2020, 2021 and 2022 was equivalent to a historical 96th percentile, 94th percentile rainfall and 97th percentile, respectively (very wet). Rainfall during 2023 was low, equivalent to a historical 28th percentile (dry). In 2024, elevated rainfall was observed at the site with an annual total amounting to 81st percentile rainfall (wet).



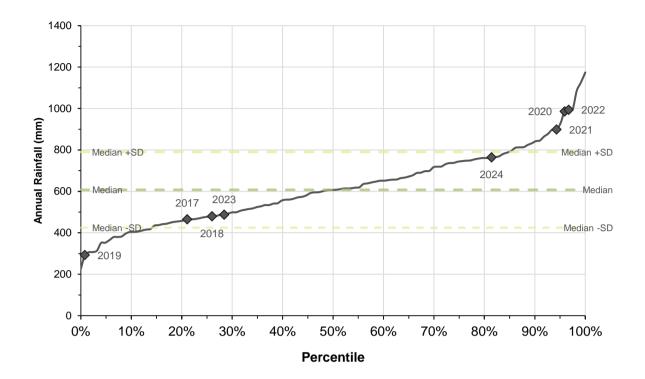


Figure 2: Historical Annual Rainfall Percentiles

### 4.2.2 Rainfall Statistics (Data Drill)

The statistics for the long-term Data Drill rainfall data for the 125-year period are summarised in **Table 5**. Annual totals are for a calendar year January to December.

Table 5: Long-Term Data Drill Rainfall Statistics (mm)

Statistic	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Max	204	364	241	200	184	249	175	137	174	216	266	203
90th %ile	132	140	122	79	74	88	97	84	90	109	122	124
Median	60	45	45	32	31	34	40	37	35	51	56	50
10th %ile	14	5	5	2	5	10	7	12	10	9	10	12
Min	0	0	0	0	0	0	1	0	0	0	0	0
Mean	67	62	57	40	38	46	45	44	43	53	61	62
St. Dev	45	59	49	37	33	41	33	29	32	42	47	46
Count	125	125	125	125	125	125	125	125	125	125	125	125

#### 4.2.3 Data Drill vs Site and BoM Rainfall

SILO Data Drill rainfall data has been compared against data recorded at the WCM AWS and at the neighbouring BoM rainfall gauge at Wollar (BoM station ID 062032, approximately 8 km east of Wilpinjong).

The intent of comparing SILO Data Drill rainfall against the site and BoM reference data is to:



- Demonstrate that the SILO rainfall is comparable to local measurements, and is therefore an appropriate input time series to the Wilpinjong WBM model (for longterm modelling); and
- Identify an appropriate measured rainfall dataset to be used in the WBM calibration exercise completed as part of current investigations.

Cumulative rainfall totals, resetting on an annual basis, are presented in **Figure 3**. Note the BoM rainfall gauge at Wollar lacked data for April 2024, reducing total annual rainfall when compared to the SILO Data Drill and WCM AWS. If the April average from these stations (68 mm) is assumed for the BoM rainfall gauge, annual rainfall at the gauge totals 744 mm.

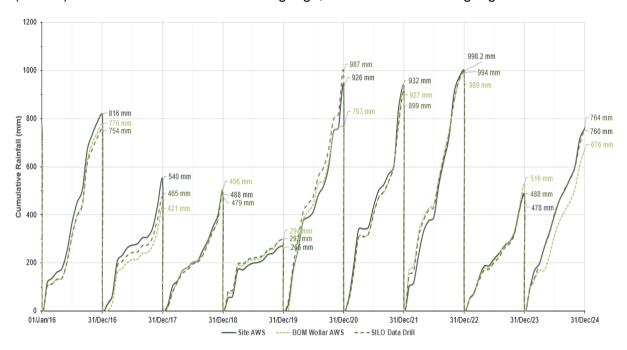


Figure 3: Cumulative Rainfall (Resetting 1st Jan) - Site AWS, BoM Wollar, SILO

Data from the AWS appears to be consistent with the other stations. Note the 2016 model update (Hatch, 2017) compared Site AWS data against data from nine surrounding BoM rainfall gauges (including Wollar) and observed similar trends in 2014 and early 2015.

Key outcomes of the above comparison are:

- The model calibration exercise completed as part of this update has focused on the period January 2018 to December 2024 (seven years). The first year of this period overlaps with the calibration period studied as part of previous investigations (WRM, 2019). For consistency with previous model updates, model calibration was based on the Site AWS data; and
- SILO Data Drill rainfall is consistent with data recorded at gauges in the study area and is therefore considered to be an appropriate input time series to the WBM.

### 4.3 Evaporation

Long-term daily evaporation data for the WCM has been sourced from the SILO Data Drill service. Morton lake evaporation ('Mlake') has been used to estimate evaporation from the wet surface areas of surface storages.

No adjustment factors have been applied to pits or catchment areas. The statistics for the long-term Data Drill Mlake evaporation data are summarised in **Table 6**.



Table 6: Long-term Data Drill Mlake Evaporation Statistics (mm)

Statistic	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Max	229	186	164	108	67	45	53	84	122	165	204	232	1,539
90th %ile	217	173	151	98	62	42	50	76	112	157	186	213	1,461
Median	196	156	137	90	56	38	44	69	102	142	168	193	1,393
10th %ile	169	138	125	81	50	33	39	62	91	126	149	176	1,300
Min	153	122	106	67	44	30	33	58	79	101	109	149	1,135
Mean	194	156	136	89	56	38	44	69	102	142	168	193	1,387
St. Dev	17	14	11	7	5	3	4	6	8	12	15	15	65
Count	125	125	125	125	125	125	125	125	125	125	125	125	125

### 4.4 Catchment Yield

### 4.4.1 Overview

Accurate estimation of catchment yield hydrology is an important component of water management investigations. Catchment yield within the WBM is simulated using the Australian Water Balance Model (AWBM). The AWBM is a saturation overland flow model which uses daily rainfalls and estimates of catchment evapotranspiration to calculate daily values of runoff using a water balance approach (Boughton, 1993). The AWBM is widely accepted and commonly used throughout Australia.

#### 4.4.2 Parameters

Different AWBM model parameters are defined for each land use type within the mine catchment. AWBM model parameters were initialised using values from the previous 2019 model update (WRM, 2019) and are considered to remain well suited to current site conditions, determined through the WBM calibration.

Adopted AWBM model parameters are summarised in **Table 7**.

**Table 7: Calibrated AWBM Parameters** 

Parameter		Natural	Rehab	Spoil	High Runoff (Hardstand/Active Pit)
	A1	0.134	0.134	0.134	1.0
Partial Areas	A2	0.433	0.433	0.433	-
	А3	0.433	0.433	0.433	-
	S1	17.6 mm	14.7 mm	11.0 mm	17.0 mm
Soil Storage	S2	182.6 mm	153.2 mm	114.1 mm	-
	S3	366.2 mm	306.9 mm	228.8 mm	-
Baseflow Index	BFI	0.50	0.50	0.50	0.00
Surface Lag	Ks	0.80	0.97	0.97	0.00
Baseflow Lag	Kb	0.97	0.80	0.80	0.00
Avg. Storage	S_avg	239.9 mm	201.2 mm	150.0 mm	17.0 mm



# 5.0 Site Water Usage

# 5.1 CHPP and MIA Usage

Water is pumped from Pit 2W to the RWD and CWD. Water is then pumped from these dams into a distribution network which is used to supply water to the following demands within the CHPP and MIA area:

- CHPP process;
- MIA wash-down pads;
- Heavy vehicle (HV) and light vehicle (LV) wash bays;
- Coal handling/stockpile dust sprays; and
- Other miscellaneous MIA/CHPP tasks (cleaning/hoses, clarifier tank overflow or bleed-off via old tailings lines).

Water supply from the RWD and CWD to the distribution network is metered, but the individual offtakes are unmetered (WRM, 2019).

The following sub-sections summarise a process which has attempted to separate the CHPP process water makeup from the other MIA demands.

## 5.1.1 CHPP Usage

#### **5.1.1.1** Overview

A conceptual model of the coal washing process is shown in **Figure 4**. Note that prior to April 2015 the CHPP reject circuit comprised separate coarse and fine waste material streams. Coarse rejects were trucked and disposed of within in-pit overburden dumps, and fine tailings were pumped as a slurry to tailings cells adjacent to Pit 2W. The CHPP tailings circuit was modified in April 2015 to include a BFP, which dewaters the tailings stream and allows this material to be disposed of as a dry waste stream with the coarse reject. Any moisture bleed-off from within the BFP process is captured and re-circulated to the clarified water tank. Excess water from the clarified water tank may be drained off by pumping water to the tailings dams via the old slurry pipelines (WRM, 2019).

The following moisture contents are assumed for various material streams within the CHPP:

ROM: 5% moisture w/w
Bypass coal: 7.5% moisture w/w
CHPP feed: 7.5% moisture w/w
Product coal: 10.3% moisture w/w
Mixed reject: 28.0% moisture w/w



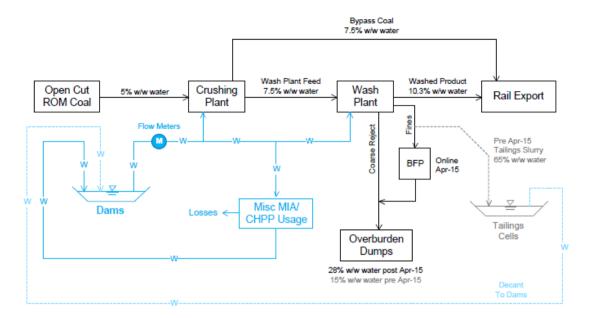


Figure 4: Coal Washing Process Conceptual Model (Source: WRM, 2019)

#### 5.1.1.2 Historical Production

Material tonnages have been summarised in **Table 8** for the 2021 through 2024 calendar years alongside forecast production for 2025. Review of **Table 8** shows that the annual railed product was approximately 10.97 Million tonnes (Mt) in 2024 which is an increase upon 2023 production. This is the first year-on-year increase in railed product since 2021.

**Table 8: Production Summary** 

Material Stream	2021	2022	2023	2024	2025 (Forecast)
Waste Rock/Overburden	43.71 Mbcm	40.31 Mbcm	40.45 Mbcm	41.08 Mbcm	53.86 Mbcm
ROM coal^	14.48 Mt	13.28 Mt	12.84 Mt	13.28 Mt	13.90 Mt
Coarse Reject & Tailings (BFP)	2.57 Mt	2.20 Mt	2.11 Mt	2.90 Mt	2.30 Mt
Fine Tailings	0	0	0	0	0
Railed product	12.17 Mt	11.05 Mt	10.53 Mt	10.97 Mt	11.56 Mt

**Note:** ^WCM are approved for a rate of up to 16 Mtpa out to 2033.

Mbcm: Million bulk cubic metres.

# 5.1.1.3 Process Water Makeup

**Figure 5** presents the metered water supply from the RWD and CWD to the CHPP-MIA water distribution network. Data relating to the allocation of water to the CHPP area and MIA separately were not recorded for the 2019, 2023 or 2024 monitoring periods.



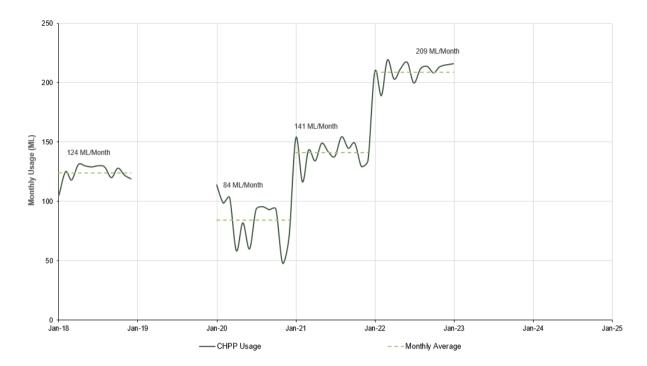


Figure 5: CHPP and MIA Monitored Demand

Review of Figure 5 shows the following:

- No CHPP or MIA water usage data was recorded during 2018, 2023 or 2024;
- Based on advice from WCM, a throughput of 3 ML/day (~92 ML/month) at the CHPP has been assumed within the WBM for the 2024 reporting period;
- A rate of 140 ML/month has been adopted for modelling of the 2023 reporting period, taken as the average of all monitored usage to date;
- Water usage, when last metered in 2022 at an average of 209 ML/month, was on a continually increasing trend from 2020;
- The water supply rate for 2021 fluctuated between 116 ML/month and 154 ML/month throughout the year. The average monthly usage rate was 141 ML/month;
- Combined CHPP water usage for 2020 was an average supply rate of 84 ML/month which is significantly less than 2021; and
- Combined CHPP water usage for 2018 was an average water supply rate of 124 ML/month. No water usage data is available for 2019.

## 5.1.1.4 Model Configuration

The yearly fluctuation in water demand at the CHPP has been set as a time series with demand for each year set as the average monthly usage for that year (i.e., 209 ML/month for 2022). As no data was available for the 2024 reporting period, a usage of 92 ML/month has been adopted based on advice from WCM.

Note the model assumes all water sent to the CHPP to close the mass balance is lost, with nil recovered (e.g., all water is entrained within railed product or in-pit dumps). Note that a 20 ML/month miscellaneous usage is modelled with a large percentage of this water returning to Pit 2W (see **Section 5.1.2**). It is possible that a portion of this water is associated with activities in the CHPP.



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## 5.1.2 MIA and Miscellaneous Usage

Previous model updates have shown an unaccounted-for component of the RWD and CWD water supply which is estimated at approximately 20 ML/month. This flow rate is understood to represent water supply to the various demands listed in **Section 5.1.** 

Based on the previous water balance modelling, the inferred net loss rate from this miscellaneous water usage stream is expected to be relatively low. Modelling has adopted a net water loss of 100 ML/year (8.3 ML/month) which is consistent with the previous 2019 model update (WRM, 2019) and typical MIA water consumption observed at other operations comparable to Wilpinjong.

The WBM has been configured to extract 20 ML/month from the CWD or RWD and recirculate 17.4 ML/month of this flow back into the WMS via Pit 2W.

# 5.2 Haul Road Dust Suppression

## 5.2.1 Measured Water Usage

Water is extracted from the WMS and applied using water trucks over HV/LV roads to minimise dust lift-off. There are four fill points (FPs) in operation: the ROM FP, Pit 2 FP, Pit 3 FP, and Pit 5 FP. All water truck fill points have been fitted with flow meters.

From 2019 to 2021, the site utilised Dust-a-side (DAS) at the site to help minimise water usage associated with dust suppression. DAS is a dust suppression agent that reduces dust generation on roads, hardstand and laydown areas and reduces the need for water carts. At a stage in 2021, WCM moved to a Reynolds Soil Technologies (RST) product to assist with dust mitigation and reduce dust suppression water usage.

On the occasion that FP flow meters are offline or technical malfunction occurs, and daily data cannot be obtained, trip-count data is used to estimate usage. WCM operates a Global Positioning System (GPS) logging system which maintains a count of how many times each truck has driven within a certain proximity of a FP. Water usage is estimated by multiplying each individual truck's trip count by its respective water fill capacity.

WCPL have provided updated flow meter data and trip-count-based estimates of water usage for January 2016 to December 2024. **Figure 6** presents haul road dust suppression data throughout this period. Across 2023, no data pertaining to water truck usage was recorded by the site.

It has been assumed that actual haul road dust suppression water losses are lower than what is recorded by the flow meters and/or estimated based on GPS trip counts. Consistent with previous model updates an adjustment factor of 0.9 has been applied to the historical water usage data to account for the following:

- Flow recirculation recorded by flow meters (e.g. trucks being overfilled, with excess water draining back to the supply dam); and
- Over-estimation bias inherent to trip-count based methods, which assume every "trip" entails a truck being filled from empty to full, whereas in practise trucks may return to the fill point part-full or may even drive past the fill point without stopping (which is still registered as a "trip").



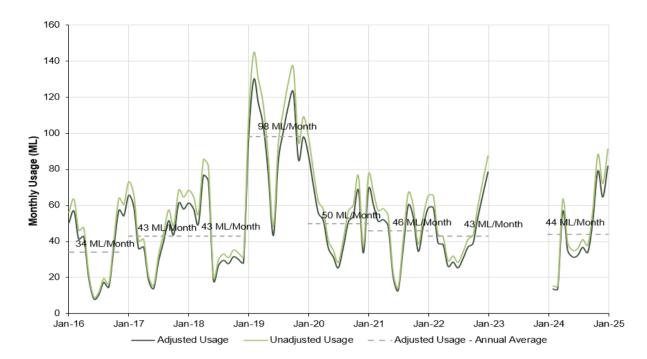


Figure 6: Metered Haul Road Dust Suppression Water Usage

Review of **Figure 6** indicates:

- Water usage is seasonal, with highest usage rates occurring in summer, and lows in winter. Seasonal variability is driven largely by changes in ambient temperature and evaporation rates;
- Water usage is also lower during periods of rainfall; and
- Average water usage rates during 2016-2018 are relatively consistent year-to-year at around 34-43 ML/month (408-516 ML/year), however, 2019 usage was significantly higher than previous years. This is likely to be attributed to the prevalent drought conditions experienced throughout 2019 including limited rainfall and increased evaporation. Water usage during 2020, 2022 and 2024 was reduced and closely followed pre-2019 levels with annual averages between 43-50 ML/month corresponding to elevated rainfall in each of these years.

No data breakdown of dust suppression demand by FP was provided for 2019 – 2024 and it is therefore assumed to be consistent with 2018 values discussed in the 2019 update (WRM, 2019). The breakdown by FP in 2018 is as follows, noting data is yet to be available for the Pit 3 FP installed in late 2023:

ROM FP: 75.08%Pit 2 FP: 24.91%Pit 5 FP: 0.01%

# 5.2.2 Dust Suppression Sub Model

Haul road dust suppression water usage is simulated within the WBM using a sub-model, which accounts for the seasonal variation and sensitivity to rainfall observed in the metered usage data. Daily water application is calculated as a function of wetted haul road area, evaporation, and rainfall. Water is applied to offset daily evaporation from the wetted area. Evaporation rates are subject to monthly adjustment factors. Application is cancelled if rainfall exceeds a nominated minimum threshold of 1.5 mm/day (WRM, 2019).



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Monthly evaporation factors and the rainfall threshold determined in the previous model update are compared to measured water usage rates during the period January 2018 to December 2024 and adjusted as required. The results of this process are presented in **Figure 7**. Note that measured data have been factored per **Section 5.2.1**.

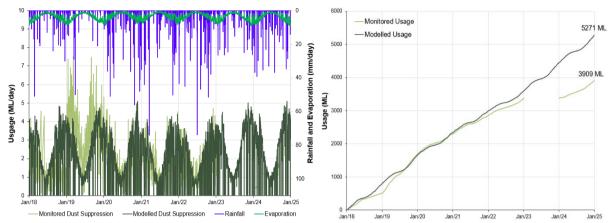


Figure 7: Dust Suppression Sub Model: Modelled vs Monitored Data

Review of **Figure 7** shows relatively good agreement between calculated and measured data. Anomalies do occur throughout the calibration period however overall usage shows good correlation with seasonal trends demonstrated. Results have been derived using the following parameter set consistent with the 2019 model update (WRM, 2019):

Haul road wetted area:
 44 ha (per WEP surface water assessment, WRM 2015)

Rainfall threshold: 1.5 mm/day

Evaporation adjustments:

January to February: 1.1
March to June: 1.6
July to September: 1.9
October: 1.7
November: 1.5
December: 1.3

The parameter adjustment process has sought to reproduce: 1. total usage volumes, 2. seasonal variation in water usage (i.e. general peaks and troughs in spring/summer and autumn/winter respectively), and 3. sensitivity to rainfall (reductions in usage during wet periods such as winter 2016, 2020 and 2022).

Additionally, monthly adjustment factors are the same for each year and should also follow a relatively smooth profile within the year (e.g., not fluctuating up and down repeatedly).

# 5.3 Water Destruction (Sprays)

WCPL have access to a suite of evaporator sprays which are employed to reduce water inventory in times of surplus. The site operates the sprays at two distinct locations:

- On the eastern bank of Pit 2W; and
- On the southern bank of Pit 3.

The sprays at Pit 2W have been intermittently run since 2017, while the sprays at Pit 3 were only recently introduced during 2024. Between October 2017 and February 2018, a total of



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ten sprays were in operation at Pit 2W. Water supply to the spray system was unmetered and has been estimated at approximately 1 ML/day. Net water losses have been estimated at 0.25 ML/day assuming a 25% spray efficiency, which has been selected based on past experience with similar systems at other operations. The Pit 2W sprays were not operated from 2019 – 2021, then later recommissioned in 2022 and 2023 and employed on an ad-hoc basis. In 2024, the Pit 3 sprays were run during late December with a total usage volume of 38.6 ML reported.

The WBM has been configured to model a net 0.25 ML/day water extraction from Pit 2W. The outflow is assumed to remove no salt from Pit 2W. In addition, a net 0.125 ML/day water extraction from Pit 3 has been included within the WBM from 1 July 2024 onwards based on discussions with WCPL during water forecasting works completed in late 2024 (SLR, 2024b). This outflow assumes no salt loss in Pit 3. Therefore, a cumulative 0.375 ML/day loss to evaporator sprays is modelled within the WBM after 1 July 2024. The 38.6 ML recorded throughput to the Pit 3 sprays was incorporated in the WBM, amounting to a 9.65 ML reduction in Pit 3 during late December 2024 assuming 25% spray efficiency.

Operation of the spray system has been assumed to cease if the combined inventory in the WMS is reduced below a specified minimum threshold, which was initially specified at 1,000 ML in previous model iterations. This threshold was increased to 4,000 ML in the 2023 model update to better reflect site operations during drier periods and observed operation during wet periods. This threshold is considered suitable for continued use in this model update; however, this threshold should be reviewed and accordingly revised where new data indicates the need.

# 5.4 Harvestable Rights

The site is located within the coastal draining catchments and central inland-draining catchments harvestable rights area. As of September 2023, up to 10% of the average annual regional rainfall runoff may be captured and used for any purpose within this harvestable rights area, as per the *Harvestable Right (coastal-draining catchments) Order 2023* (DPE, 2023) under the *Water Management Act 2000*.

The WCPL landholding area is 20,400 ha. Using a harvestable rights multiplier of 0.07 as per the Department of Planning and Environment (DPE) guidelines, the harvestable right for the site is 1,428 ML. Based on rainfall data sourced from the Site AWS, the annual rainfall for the reporting period is 760.4 mm (refer **Section 4.2**).

There are currently 423 farm dams located within the WCPL landholding area. Due to the nature of these dams, the capacity is unknown. The method set out by DPE to determine the capacity of existing harvestable rights dams for small dams (less than 10 ML) has been used to determine the approximate capacity of WCPL farm dams within the land holding. The capacity of these existing water storages is estimated at approximately 242 ML.

It is noted that the maximum harvestable right does not include storages that are used to control pollution to a water source. Dams used solely for the capture, contamination, and recirculation of drainage and/or effluent, consistent with best management practices or required by a public authority to prevent the contamination of a water source, that are located on a minor stream are exempt under *Clause 3 of Schedule 1 of the Water Management (General) Regulation 2018.* 

For the site, preventing contamination includes the capture of predominantly "dirty" water, including sediment laden runoff and mine water runoff. Therefore, water that is captured within mining disturbance boundaries is exempt from requiring a Water Access Licence (WAL), water use approval or water supply work approval. Water captured can be used for any purpose, such as dust suppression and processing, provided it does not result in the contamination of a water source. This definition applies to all the site's water storages, with



the exception of Pit 8 CWD which has a predominantly vegetated contributing catchment, meaning this dam captures "clean" rainfall-runoff. As such, the storage volume of Pit 8 CWD (25 ML) has been included under the site's harvestable right allowance.

The current mining disturbance area captured within the site water management system is 2,579 ha. Clean water catchment draining internally to the mine water management structures consists of 1,259 ha. The estimated runoff captured from these clean water areas is 1,021 ML.

The total WCPL harvested volume is calculated as:

Farm Dam Capacity + Pit 8 CWD + Clean water draining WCM = Total Harvested Volume (ML)

The calculated volume is described in **Table 9**.

Table 9: Harvestable Rights Inputs and Estimates – 2024 Reporting Period

Parameter	Input Value
Annual Rainfall Depth (mm)	760.4
Runoff Coefficient (clean catchment)	0.11
Mine Disturbance Area (ha)	2,579
Clean Catchment Draining to WCM (ha)	1,146
Storage / Licence	Estimated / Known Value
Clean Water to WCM (ML)	1,021
Farm Dam Capacity + Pit 8 CWD (ML)	267
WAL Volume (ML)	150
Reporting Volume	Estimated Value
Total Harvested Volume (ML)	1,288
Surplus Volume (ML)	140
Surplus Volume (with WALs) (ML)	290

The total harvested volume for 2024 is estimated to be 1,288 ML. Given that the WCPL harvestable right is 1,428 ML, the site was within its harvestable right allowance and had a surplus allowance of 140 ML for the year.

Additionally, WCPL hold 150 ML in WALs. Therefore, the site had a surplus allowance of 290 ML including these WALs during 2024.

# 5.5 Water Accounting

WCPL undertake accounting related to water usage at WCM in order to quantify the various water input and output streams associated with the site. This reporting assists WCPL to appreciate how water enters and exits the operation, the quantities and qualities of different streams, compliance against licensing, as well as measuring how effective the site is at sustainably managing water through reuse and recycling practices.

**Table 10** presents estimated accounts of water usage at WCPL for the 2024 reporting period. These estimates are based upon a combination of monitored inputs at the site and estimates made using tools including GIS, WBM and the numerical groundwater model.



Table 10: Water Accounting at Wilpinjong - 2024 Reporting Period

Metric Code	Item	Estimated Volume (ML)	Description
M110	Fresh surface withdrawal (<1,000 TDS)	1,288	Estimated based on harvestable rights calculations
M175	Other withdrawal (groundwater)	722	Groundwater model outputs (SLR, 2020)
	(>1,000 TDS)		Permian Aquifer Take – 550 ML
			The Permian aged Illawarra Coal Measures aquifer generally has TDS >1,000 mg/L
			Alluvial Aquifer Take – 172 ML
			Wilpinjong Alluvial Aquifers (Wilpinjong, Cumbo and Wambo Creeks) generally have TDS >1,000 mg/L
M195	Fresh withdrawal third-party	4.23	Potable Usage
M236	Discharge (Surface)	1,569.6	RO Discharge to Wilpinjong Creek
	Water Reuse	2,790.8	Captured water reused for demands onsite.
			<b>Dust Sup</b> . – 590.6 ML – estimated based on trucking rate.
			CHPP – 1,460 ML – estimated based on average CHPP usage in WBM
			MIA/Misc – 240 ML – estimated based on WBM
			RO Reject - 500.19 ML - Measured
	Water Recycled	1,569.6	RO Discharge to Wilpinjong Creek



# 6.0 Water Treatment Facility

### 6.1 Overview

WCM operate a Water Treatment Facility (WTF) which is used to treat excess mine water and discharge a blend of permeate and mine water to Wilpinjong Creek in accordance with conditions outlined in EPL 12425. The WTF comprises a reverse osmosis (RO) treatment plant which is permitted to release at a rate of 6.5 ML/day as of March 2023.

For the period between January 2017 and January 2018, a secondary RO treatment plant leased from General Electric (GE) was in operation, increasing the prescribed maximum release rate to 15 ML/day. The second RO treatment plant was decommissioned at the beginning of 2018 once the site's mine water inventory had been sufficiently reduced. Following decommission, the capacity of the WTF reverted back to the original capacity of 5 ML/day. Due to considerable drought conditions experienced during 2018 and 2019, the RO treatment plant was decommissioned for the period between November 2018 and November 2020. The RO plant was recommissioned following considerable rainfall throughout 2020 resulting in significant surplus water within the site inventory.

Current license conditions require a maximum release water electrical conductivity of 500  $\mu$ S/cm, a pH range between 6.5 and 8.5, oil and grease not to exceed 10 mg/L and total suspended solids not to exceed 50 mg/L.

The WTF is located adjacent to and east of Pit 2W (location marked in **Figure 1**). Feed water is extracted from Pit 2W (EC 3,500 to 4,000  $\approx \mu$ S/cm), and then passes through a process of strainers, ultrafiltration filters and RO membranes to produce a low EC permeate stream (typically  $\approx$ 180  $\mu$ S/cm). The permeate stream is blended with a small amount of feed water prior to release to achieve a mixed EC closer to the 500  $\mu$ S/cm limit prescribed in the EPL. The EC of the RO reject by-product varies depending on permeate recovery but is typically around 14,000  $\mu$ S/cm EC. Prior to Q4 2018, reject was pumped to Pit 1S. Reject is now pumped to either the RWD or Pit 2W given that Pit 1S has been taken offline (mined through). Some permeate is also used for RO back-flushing/cleaning.

A conceptual schematic of the WTF and river discharge process is presented in **Figure 8** (based on the configuration prior to Q4 2018).

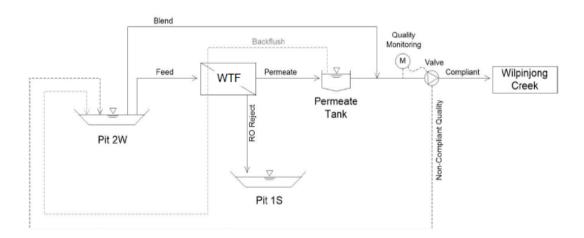


Figure 8: Conceptual Schematic – WTF and River Discharge Process (Configuration Prior to Q4 2018 (Source: Hatch, 2017)



The WCPL WTF is approved to produce enough permeate to discharge a blended stream of water to Wilpinjong Creek at up to 6.5 ML/day. With both the WCM and GE WTFs operating, the combined rate of discharge had the capacity to reach up to approximately 8 ML/day. The site's discharge capacity was then effectively reduced to zero from late 2018 to late 2020 with the decommissioning of both RO plants. The capacity of the WCPL WTF then returned to 5 ML/day from July 2021 due to significant rainfall experienced at WCM throughout 2020 and 2021. During March 2023, the WTF was approved to discharge up to 6.5 ML/day in line with an update to the site EPL criteria.

## 6.2 Historical Performance

WCPL have provided records of daily discharge volumes to Wilpinjong Creek (from both plants) for the period January 2016 to December 2024. This data is presented in **Figure 9**.

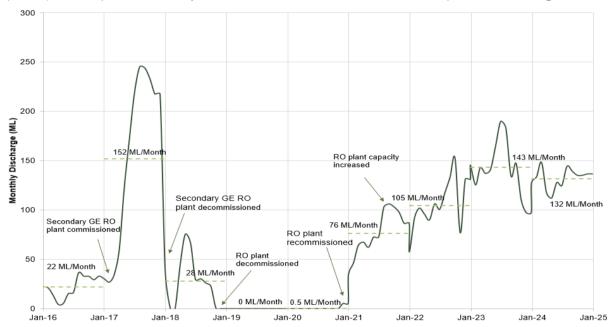


Figure 9: Historical WTF Discharge Volumes

Review of Figure 9 shows the following:

- Discharge volumes significantly increase after March 2017, following a significant wet period, modification of the Site's EPL discharge limit, optimisation of the WCPL WTF, and installation/ramp-up of the GE WTF;
- Slightly higher discharge volumes in 2018 compared to 2016, given a comparable WTF configuration. However, it is understood that the WCPL WTF was upgraded/optimised in 2016 to rectify performance problems associated with out-of-spec feed water.
- The WTF facility was not operated during 2019 and majority of 2020 due to low levels within the site inventory and very low rainfall throughout 2019;
- Discharge volumes increased in July 2021 following an increase in WTF discharge capacity after significant rainfall in 2020 and 2021;
- Significant discharge occurred throughout 2021 to 2023 as a result of significant rainfall over consecutive years from 2020 to 2022;
- Average monthly discharge to Wilpinjong Creek, while slightly reduced compared to 2023, remained high in 2024 in attempt to relieve surplus water inventory at the site.



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# 6.3 Model Configuration

The WBM has been constructed to be used for future studies with the following defined as part of the previous model updates, assuming the GE plant is offline:

- WTF capacity: up to 4 ML/day;
- Permeate recovery: 75% of feed;
- Permeate EC: 180 μS/cm;
- Reject EC: calculated in model based on feed water EC;
- Discharge water EC: 350 μS/cm EC (per recent historical sampling see **Table 16**);
- Blend water volume: assumed 0.3 ML/day based on average feed water EC and required discharge EC; and
- Assumed no reduction in RO recovery due to increasing feed water EC.

As part of the previous model updates, a set of operating rules were established within the WBM which aim to reflect on-site decisions regarding the WTF for use in future studies. These updates included adjustment of the WTFs deactivation trigger to 2,000 ML rather than the previously adopted 1,000 ML in the WRM (2019) WBM, and incorporation of the relationship between climatic conditions (i.e., rainfall) and feed water flow. These changes were assessed as part of this model update. While the WTF is approved to discharge up to 6.5 ML/day which equates to 8.7 ML/day of feedwater, review of observed discharge data from late 2020 (when the RO plant was recommissioned) to end 2024 illustrates the site discharges well below their allowance at an average of 3.7 ML/day. As such, the operating rules have been revised within this model update (**Table 11**) to improve the fit of observed versus modelled discharge from the WTF as well as modelling of Pit 2W.

Operation of the WTF is based on both site mine water inventory and rainfall forecasts. From historical monitoring data it is also observed that discharge flows vary and may not always operate at full capacity. Due to limited software capabilities, predicting rainfall beyond the current timestep cannot be determined. Rather, daily feed water flows within the WBM are determined by the previous 5-day rainfall and the level within the site mine water inventory. Application is cancelled if site inventory exceeds the nominated minimum threshold of 2,000 ML.

Inflow rates to the WTF have been based on discharge flows and their associated rainfall and site inventory levels given in the January 2018 to December 2024 monitoring data. The results of this process are shown in **Figure 10**.



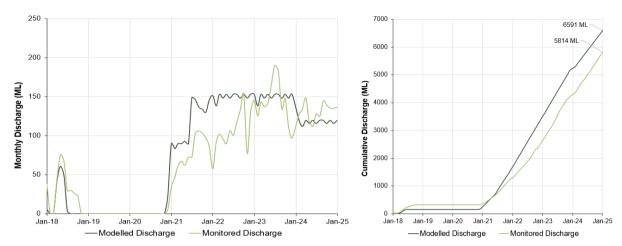


Figure 10: WTF Sub Model: Modelled vs Monitored Values

Review of **Figure 10** shows relatively good agreement between calculated and measured data excepting the 2021-22 period. Results have been derived using the relationship described in **Table 11.** 

Table 11: Feedwater Flow Rate Relationship

Site Inventory (ML)	5-Day Rainfall (mm)	Feedwater Flow (ML/day)
>3000	-	4.0
2000 2000	>1.5	4.0
3000 - 2900	≤1.5	3.8
2900 - 2800	>1.5	3.7
2900 - 2000	≤1.5	3.3
2800 - 2700	>1.5	3.5
2000 - 2700	≤1.5	3.1
2700 2600	>1.5	2.9
2700 - 2600	≤1.5	2.0
2600 - 2500	>1.5	2.8
2000 - 2500	≤1.5	0.9
2500 2400	>1.5	2.5
2500 - 2400	≤1.5	0.8
	>20	0.9
2400 - 2350	20 – 1.5	0.7
	≤1.5	0.3
2350 - 2000	>20	0.3
2330 - 2000	≤20	0
<2000	-	0

The WTF operating rules have sought to better simulate inflows and associated outflows for the WTF based on climate variation and site inventory levels for use in predictive studies. The WBM has been verified with seven years of data and should continue to be refined and validated using observed site data.



# 7.0 Discharge

# 7.1 Controlled Discharge

In 2021 following the development of Pit 8, WCPL sought a variation to EPL 12425 to allow the clean water collected by the diversion upstream of Pit 8 to discharge to Wilpinjong Creek under various water quality conditions. The approved LDP 30 permits water to be discharged from the Pit 8 CWD if the value of turbidity does not exceed the turbidity value measured at the Wilpinjong Creek upstream gauging station. When there is no flow within Wilpinjong Creek at the upstream gauging station, the value of turbidity measured at point LDP 30 must not exceed 50 Nephelometric Turbidity Units (NTU).

Monitored controlled discharge from Pit 8 CWD is shown in **Figure 11**. There has been no discharge recorded from LDP 30 since early 2023.

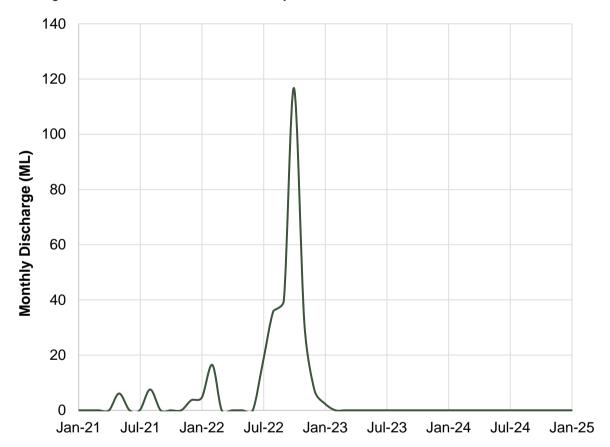


Figure 11: Pit 8 CWD Controlled Discharge to LDP 30

## 7.1.1 Model Configuration

The WBM has been configured to model a net 17.3 ML/day (200 L/s) water extraction from Pit 8 CWD. Discharge occurs when the volume in the CWD is above 80% full.



# 7.2 Emergency Discharge

In October 2022, WCPL sought an exemption under S.284 of the *Protection of the Environment Operations Act* (POEO Act) to allow for the emergency off-site discharge of mine water due to above average rainfall associated with the third consecutive La Nina year.

The total cumulative annual rainfall recorded for 2022 was 987 mm, for 2021 was 942 mm and for 2020 was 916 mm. This represents three consecutive years of annual rainfall above 90<sup>th</sup> percentile annual rainfall.

Licence Variation Notice 1623919 to discharge from the premises, under emergency conditions, from surplus rainwater captured and stored in open cut pits and associated dams was approved on 31 October 2022. *Licence Condition E1 Emergency Water Discharge* permitted discharge from several LDPs up to the following volumes:

- EPL daily discharge limit of 71 ML, including:
  - LDP 30 18 ML/day
  - LDP 31 18 ML/day
  - LDP 32 35 ML/day

Emergency discharge occurred from 31 October 2022 to 25 November 2022 (Phase 1) and 15 December 2022 to 1 January 2023 (Phase 2). Phase 2 imposed limits of 90 ML and 270 ML from LDP 30 and LDP 32, respectively. A summary of water discharged is given in **Table 12**. A total of 1,607 ML was discharged between 31 October 2022 and 1 January 2023. No emergency discharges were reported for the remainder of 2023 or for 2024.

**Table 12: Summary of Emergency Discharge** 

Discharge Point	Daily Average (ML)	Daily Maximum (ML)	Daily Limit (ML)	Total Volume (ML)	Permitted Under EPL
Phase 1					
LDP 30 (Pit 8)	11	14	18	243	
LDP 31 (Pit 4)	15	16.6	18	389	71 ML/d
LDP 32 (Pit 2W)	25.2	33.5	35	655	
Phase 2					
LDP 30 (Pit 8)	4.65	4.95	5	85	90 ML
LDP 32 (Pit 2W)	13.05	15.32	15	235	270 ML

## 7.2.1 Model Configuration

Due to the nature of the discharge and the requirement for changes to approvals, emergency discharge is not included in the WCPL WBM. Rather model results have been adjusted following calibration to account for the emergency release of water from the water management system as described in **Section 11.3**.



# 8.0 External Water Import

## 8.1 Overview

WCM have access to external water supply bores that are operated as required. Given the historic surplus mine water in storage at the site, WCM did not require this source until the extreme drought conditions that occurred during 2018 and 2019. External water was sourced from the water supply bores during May 2019 to March 2020. Accessible external water supply sources are outlined below:

- WCM water supply system includes an external water supply borefield;
- It is understood that WCPL are licensed to collectively take up to 3,121 ML annually (equivalent to 8.55 ML/day) including water pumped from mining pits, inferred groundwater and water supply bores;
- WCPL has an in-principle agreement with the nearby Moolarben Coal Mine to source excess water from this mining operation (by pipeline) if required in the future (subject to approval); and
- Local contractors that truck potable water to site for staff amenities (does not enter the WMS).

WCM have provided records of water import volumes for the January 2019 to December 2024 period. **Figure 12** shows the recorded water import volumes which solely accounts for water imported from the external borefield. It is understood the borefield has not been utilised since 2020 due to adequate water holdings/supply on site.

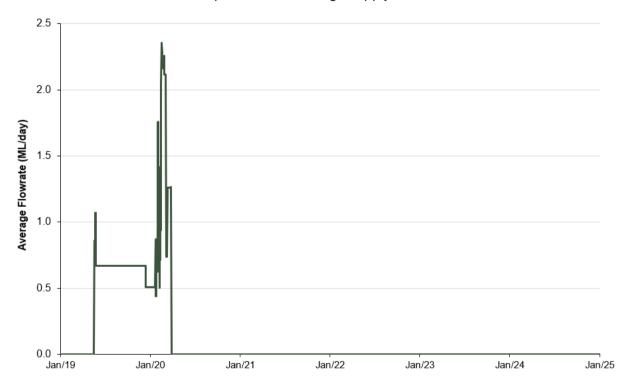


Figure 12: Average Daily Import Rates

Review of **Figure 12** shows a consistent supply of water from 17 May 2019 to 26 March 2020 with an average flowrate of 0.67 ML/day (7.8 L/s) in 2019 and 1.16 ML/day (13.4 L/s) in 2020. Based on the 2019-2020 monitoring data, a maximum of 27.3 L/s was supplied to the mine via the external borefield. Since 2020, the external borefield has not been operated.



# 8.2 Model Configuration

The WBM has been configured to import water from an external source if the combined mine water inventory falls below a specified minimum threshold. This threshold was increased from 500 ML in the WRM (2019) update to 2,000 ML in the previous model updates to reflect observed operations during dry periods. Additionally, a series of pump operation rules have been established to relate the rate of external supply into the WMS to the site inventory levels.

The external supply operating rules included in the WBM are as follows:

- External water is supplied at a varying rate depending on combined mine water inventory levels;
- Benchmark values are set as:
  - Combined mine water inventory 2,000 ML assumed pumping rate of 5.1 L/s (0.44 ML/day).
  - Combined mine water inventory 1,000 ML assumed pumping rate of 9.9 L/s (0.86 ML/day).
  - Combined mine water inventory 500 ML assumed pumping rate of 27.3 L/s (2.35 ML/day).
- External water supply pump rates are linearly interpolated between the benchmark values based on the combined mine water inventory; and
- Water is assumed to be sourced from the borefield and pumped into the CWD storage, where it is then pumped on to supply tasks as required.

Modelled external supply volumes determined using the above operating rules have been compared to the measured water supply volumes during the January 2019 to December 2024 period. The results of this process are presented in **Figure 13**.

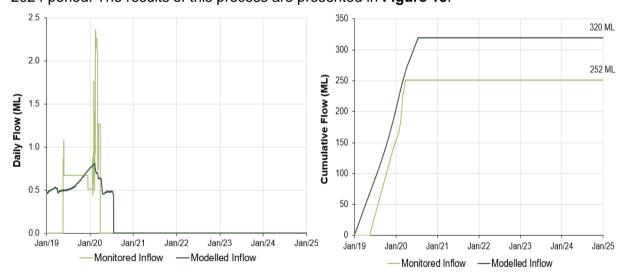


Figure 13: External Water Supply: Modelled vs Monitored Values

As shown in **Figure 13**, modelled data shows reasonable correlation to measured data where external water supply is active. Although anomalies are observed between the modelled inflows and that of the monitored data, the intent of this operation is to allow predictive studies to better determine reliance on external water sourcing. The modelled operating rules provide a more reflective simulation during dry conditions as opposed to a single threshold trigger as previously applied within the WBM.



# 9.0 Groundwater

### 9.1 Groundwater Inflows

#### 9.1.1 Definition

Groundwater inflows are defined as waters reporting to the WMS from aquifers external to the current extent of disturbance. This generally includes seepage from coal seams and insitu rock and alluvial aquifers, and water released from fractures and pores within coal and rock as it is broken as part of the mining process (WRM, 2019).

#### 9.1.2 Previous Estimates

Previous estimates of groundwater inflow to the WCM include the following:

- WEP EIS (2015): net groundwater inflow rates adopted as part of the WEP surface water assessment (WRM, 2015) were derived by applying highwall evaporative losses to gross inflow rates determined through hydrogeological modelling as part of the groundwater assessment (HydroSimulations, 2015);
- Previous 2016 model update (Hatch, 2017): net groundwater inflow rates were inferred at a constant rate of 3.8 ML/day through the period January 2014 to January 2017 as part of the water balance model calibration process;
- Previous 2019 model update (WRM, 2019): net inflow rates determined through model calibration exercise varying from 3.51 ML/day in 2014 to 2.00 ML/day in 2018;
- Previous 2020 model update (SLR, 2020a): net inflow rates determined through model calibration exercise as 1.8 – 2.0 ML/day in 2023; and
- Groundwater Model Update (SLR, 2020b): net groundwater inflow rates determined from hydrogeological modelling as 2.5 ML/day.

## 9.1.3 Current Estimates

Groundwater inflow rates have previously been inferred for a given year through historical model calibration (WRM, 2019). However, during a previous model update an operation within the model was established that varies groundwater inflow depending on the state of groundwater influences therefore allowing the model to be more effectively used as a predictive tool for determining future on-site water volumes. This operation allows groundwater inflows to be adjusted based on recent rainfall trends to align simulated mine water inventory trends during dry and wet periods. The degree to which groundwater inflows are adjusted has been determined using historical model calibration. Updated adjustment factors include the following:

- Mean 6-monthly rainfall (300 mm) correlates to the mean modelled groundwater inflow (SLR, 2020a) of 2.0 ML/day;
- 6-monthly rainfall greater than 25% of the mean correlates to a 15% increase in groundwater inflow (2.2 ML/day) to reflect increased groundwater recharge; and
- 6-monthly rainfall less than 25% of the mean correlates to a 15% decrease in groundwater inflow (1.7 ML/day) to reflect reduced groundwater recharge.

Based on the above operations, average groundwater inflows for the calibration period are as shown in **Table 13**. It should be noted that assessment of inferred groundwater take for WCM licence conditions is assessed based on the water year (period 1 July to 30 June).



Table 13: Summary of Average Daily Groundwater Inflow

	Calendar	Year	Water Year				
Period	Modelled Groundwater Inflow (ML/day)	Groundwater Model (SLR, 2020b) (ML/day)	Period	Modelled Groundwater Inflow (ML/day)	Groundwater Model (SLR, 2020b) (ML/day)		
2018	1.8	3.3	2018-2019	1.8	2.1		
2019	1.8	3.1	2019-2020	1.7	1.7		
2020	2.0	2.4	2020-2021	2.3	2.5		
2021	2.3	1.9	2021-2022	2.3	2.4		
2022	2.0	2.2	2022-2023	2.5	1.8		
2023	2.2	1.7	2023-2024	1.9	1.5		
2024	2.2	1.4	2024-2025	1.9	1.4		

Groundwater inflows in 2024 were estimated at an annual average of 1.9 ML/day for the 2024-2025 water year. This exceeds predictions made in the current groundwater model which equate to 1.4 ML/day (SLR, 2020b).

# 9.1.4 Model Configuration

The WBM has been configured to simulate a future net inflow rate based on 6-monthly rainfall trends as described in **Section 9.1.3**, reporting to the site WMS.

Note that the 2019 WBM model configuration does not include any groundwater inflow to Pit 8. Activities in the Pit 8 extraction area began during 2019, predominantly during the early stages of mining (i.e., pre-stripping) with limited pit development. It is therefore expected that Pit 8 was elevated above the groundwater table throughout 2019 hence no direct groundwater interception would have occurred. Groundwater inflow to Pit 8 was expected to occur during 2020 with the commencement of mining within Pit 8. As outlined in the Groundwater Model Update (SLR, 2020b), inflows across 2021 to 2024 were projected to be predominantly to Pit 6 and Pit 8. Mining in Pit 3 is scheduled to recommence in 2025-26, which will prompt adjustment of inflow proportions for subsequent model updates.

The model configuration for groundwater inflow is given in **Table 14**.

Table 14: Groundwater Intake Model Configuration

Year	Inflow to Pit (%)									
Teal	Pit 1/5/6	Pit 2/4	Pit 3/7	Pit 8						
2018	25	25	25							
2019	25	23	23	-						
2020	30	20	20	30						
2021	30	20		30						
2022	50									
2023		-	-	50						
2024										

Groundwater operations within the WBM are used as a preliminary tool to determine groundwater inflows, however, there remains scope to improve measurement of inflow to the pits to further validate groundwater inflow within the model. It is recommended that inflow assumptions continue to be revised as further information becomes available.



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# 9.2 Spoil Aquifers

#### 9.2.1 Overview

Mining operations have extracted coal from three distinct voids, termed Pit 1/5/6, Pit 2/4 and Pit 3/7 with the addition of Pit 8 in 2019 (refer **Section 3.1** and Figure 1). In-pit spoil placement areas have been formed within Pit 1/5/6 and Pit 2/4 for creation of most the mining landform. These in-pit placement areas are porous and highly permeable. The drainage characteristics of the spoil are such that up-dip pits (such as Pit 5S, Pit 1 and Pit 2S) do not need to be pumped out following rainfall events, as they freely drain down the dip of the coal (through the spoil) to the down-dip pits (i.e., Pit 5N and Pit 4). Pit 2W is also observed to seep at a high rate to Pit 4, through the interconnecting spoil placement areas, due to the large water level difference between these two areas. As mining commenced within Pit 8 during 2020, some groundwater interaction is expected to have taken place, however, is not expected to interact with the spoil aguifers.

Storage of water in-pit is expected to result in flow of water from the open water body into the adjoining spoil placement area, forming a saturated zone within the spoil in which significant volumes of water may be stored. In the event of a pit filling with water, leakage to the adjoining spoil aquifer will prolong the filling process, and conversely, leakage from the aquifer will prolong the subsequent dewatering process.

## 9.2.2 Properties

Spoil aquifer extents have been estimated based on comparison between end of year 2017 surface topography and deepest mined topographic survey (WRM, 2019). Spoil aquifer storage capacity is a function of the spoil extent and the spoil porosity.

The 2016 water balance model update (Hatch, 2017) adopted a spoil aquifer porosity of 30%, determined through model calibration (January 2014 to January 2017). The 2017 water balance update (WRM, 2018) extended the model calibration to include data recorded between January 2017 and December 2017, which includes the drawdown of Pit 5N and its adjacent spoil aquifer. A reduction in the spoil aquifer porosity value from 30% to 20% was found to be required. The 2018 water balance update (WRM, 2019) assumed a further reduction in the Pit 5N spoil aquifer porosity to 10% to replicate the observed rate of drawdown in Pit 5N during 2018. The 2018 water balance update (WRM, 2019) assumed values of 20% and 10% porosity for Pit 2 and Pit 4 spoil aquifers respectively. The porosity of spoil aquifers in this model update has been assumed as consistent with the 2018 values.

## 9.2.3 Model Configuration

Spoil aguifers have been modelled in the Wilpinjong WBM in accordance with the following:

- Spoil aquifers have been modelled adjacent to Pit 5N, below Ed's Lake, Pit 2W and Pit 4;
- Recharge and discharge occur to balance water levels between the pit lake and the adjacent spoil aquifer. Rates of transfer are governed by head difference but are typically in the order of 10 ML/day – 20 ML/day when flowing (model assumption);
- Pit 2W spoil aquifer drainage to Pit 4 (via Pit 4 spoil aquifer) modelled at a constant rate of ~10 ML/day;
- Storage characteristics have been modelled assuming 10-20% spoil porosity. Stagestorage characteristics have been provided for reference in Appendix D; and
- Seepage from up-dip pits into spoil aquifers, and back out into down-dip pits (e.g., Pit 5S to Pit 5N, or Pit 2E to Pit 2W), at relatively unconstrained flow rates.



# 10.0 Water Quality

Water quality sampling at WCM is undertaken at various locations with samples analysed for the standard suite of quality indicators. Monthly average measurements of EC for selected surface water locations have been summarised for 2024 in **Table 16** with long-term data provided in **Appendix C**. Note that limited EC data for the WMS dams or pits was provided from 2020 to 2024. Review of available information shows the following:

- Water circulating through the WMS has historically averaged around 3,000 to 4,000 μS /cm (see pit, dam and feed water data). Since 2023, available data indicates salinity has trended upwards at the site (Pit 2W and WTF discharge).
- The EC of water within CWD increased slightly in 2019, coinciding with input from external bore supplies;
- The EC of water within Pit 1S prior to 2018 is higher than the water in the rest of the WMS, due to inflow of RO reject. Concentrations of salt within this storage appear to have been diluted with upstream clean catchment runoff (RO reject EC sampled at 14,000 μS/cm in February 2017 compared to Pit 1S EC of around 7,850 μS/cm in October 2017).
- The EC of the blended discharge stream to Wilpinjong Creek has generally increased over time, now typically ranging around 350 to 450 μS/cm. This likely coincides with the salinity increase observed throughout the broader WMS which coincides with the site's increased confidence with the WMS and ability to effectively manage salinity. The discharge remains compliant with the 500 μS/cm EC end-of-pipe limit specified in EPL 12425.

The WBM maintains a running account of salt mass in all water storages which is equated to and reported as EC. Salt mass inflows are typically estimated by assigning salinity concentrations to runoff from various land use types, and to point water sources (e.g., groundwater, pipeline water).

Water quality model parameters were initially defined as part of the WEP surface water assessment (WRM, 2015). This water balance model update confirmed that these parameters continued to produce reasonable estimates of EC in the circulating WMS inventory (based on Pit 2W data). The current investigation has retained water quality parameters from these earlier studies.

Adopted water quality parameters are summarised in **Table 15**.

**Table 15: Adopted Salinity Generation Rules** 

Item	Salinity (EC) (µS/cm)		
Catchment Runoff Source			
Natural / undisturbed	1,600		
Roads / industrial / hardstand / pit	3,000		
Spoil / overburden / cleared	2,500		
Rehabilitated overburden	2,000		
Point water sources			
Groundwater	3,000		
External water supply (e.g., borefield)	3,000		



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Table 16: Average Electricity Conductivity (µS/cm) by Month and Sampling Location

		Monthly Dainfall			Da	ms				Pit	s				WTF		Ref	erence (Waterways)	
Year	Month	Monthly Rainfall (mm)	Pit 2W	Pit 1S	Pit 5 FP	CWD	RWD	Ed's Lake	Pit 5	Pit 2 NB	Pit 4	Pit 3	Feed	Permeate	Discharge	Concentrate	Wilp. Ck Upstream	Wilp. Ck Downstream	Cumbo Creek
2024	Jan	87	4,610												386		881	714	3,938
	Feb	79	4,570												369		793	614	5,336
	Mar	33	4,730												393		1,118	564	5,418
	Apr	69	4,570												393		1,001	623	ND
	May	62	4,560												407		919	660	4,384
	Jun	69	4,460												383		654	810	3,802
	Jul	65	4,440												407		683	953	3,519
	Aug	40	4,330												428		633	1,181	3,426
	Sep	45	4,690											37	424		731	771	3,564
	Oct	51	4,570												405		716	775	3,744
	Nov	117	4,690												382		724	572	4,672
	Dec	45	4,260												442		558	680	3,608

Note: Wilpinjong Creek and Cumbo Creek EC values are flow-weighted averages, calculated for that month. Rainfall totals were calculated based on the data obtained from the SILO Data Drill service.

ND – No Data.



## 11.0 Water Balance Model

#### 11.1 Overview

The WBM has been designed to simulate the operation of all major components of the WMS including catchment runoff, water inventory fluctuation and overflow, pump and gravity transfers, coal mining operations usage and return, climatic influence, groundwater inflow, open cut mine dewatering, external water supply, discharge of water to Wilpinjong Creek (via the WTF), and interaction with spoil aquifers.

The components of the WMS are described and quantified in the preceding report sections.

### 11.2 Model Schematisation

A representative schematic of the WBM has been provided in **Appendix A**. Review of **Figure 1A** shows the model comprises a network of interconnected nodes, where nodes represent key components of the WMS (dams, wash plant, pits, etc.).

# 11.3 Model Calibration

#### 11.3.1 Overview

The GoldSim model has been constructed to represent the operations taking place at WCM in the period 2018 – 2024, hence calibration of the model has been undertaken using the monitoring data provided by WCPL for the January 2018 to end December 2024 period. Water level data has been converted to estimates of water volume using storage characteristics as described in **Section 3.2.2**. Inventory data and water usage/discharge data have been utilised for model calibration.

The model calibration exercise specifically focused on reproducing the measured inventory in the combined WMS (Pit 2W, Pit 1S, RWD, CWD, Pit 5N, Pit 4 and Pit 3) with particular focus on behaviour of the water inventory during drought conditions experienced during 2018 and 2019 followed by recovery of the water inventory during the 2020 – 2022 wet period. The objective of the exercise was to infer or establish key model inputs and parameters, and to demonstrate that the WBM suitably replicates observed site inventory trends.

## 11.3.2 Configuration

The following inflows and outflows were hard coded into the model as time series data:

 Extraction of water from the RWD and CWD to supply demands in the MIA/CHPP area, including the CHPP and miscellaneous MIA demands (modelled as per metered stream in Section 5.1);

The following processes were simulated within the model:

- Climatic influence: evaporation, evapotranspiration, direct rainfall and catchment runoff based on daily rainfall data at the BoM Wollar station and Site AWS (refer to Section 4.2) and SILO Data Drill evaporation data (refer to Section 4.3);
- Water extraction from Pit 2W, the RWD and Pit 5 FP Dam for dust suppression (per Section 5.2);
- Transfer of water between storages, pit dewatering, etc. (refer to **Table 4**):
- Seepage from up-dip pits into down-dip pits via spoil aquifers (e.g., Pit 5S seepage to Pit 5N):



- Saturation and drainage of spoil aquifers adjacent to open cut pits (spoil aquifers modelled adjacent to Pit 5N, Pit 2W and Pit 4) (refer to Section 9.2);
- WTF inflow and outflow rates (refer to **Section 6.3**):
- Off-site discharges (refer to **Section 7.0**);
- Groundwater inflow rates (refer to Section 9.1); and
- External water supply rates (refer to Section 8.0).

The following parameters were adjusted to improve the overall agreement between simulated and observed historical WMS performance:

- WTF operating rules;
- Groundwater adjustment factors and groundwater inflow apportioning to pits;
- Incorporation of operations regarding Pit 8 CWD; and
- Other settings and configuration assumptions including:
  - Catchment and land use information described in Section 3.2.4;
  - Catchment and land use data in 2018 and 2019 based on data in the previous model updates; and
  - Stage storage updates as given in Appendix D.

## 11.3.3 Outcomes

Model simulated volumes have been compared against historical measurements in **Figure 14** for the period January 2018 to December 2024. Results have been plotted for the combined water inventory in the WMS (comprising the CWD, Pit 1S, Pit 2W, Pit 3, Pit 4, Pit 5N and the RWD).

Review of **Figure 14** shows discrepancies in modelled versus monitored inventory levels during the period July 2019 and May 2020. Investigation found a significant drop in site inventory occurred in July as a result of gaps in Pit 4 monitoring, monitoring then resumed in May 2020 resulting in a sudden spike in site inventory. To account for these discrepancies, the model results have been adjusted for the sudden loss and gain of volume associated with Pit 4 monitoring results.

Following a significant rainfall event in March 2021, a rapid increase in the site inventory was observed. This increase was seen to have caused a greater effect on the modelled mine water management system than that monitored. This response results in elevated levels modelled before levels are distributed within the water management system, and although discrepancies are shown, the general trend in water inventory remains consistent following this event. Following incorporation of the capacity increase of the WTF event the modelled inventory returns to similar levels to that monitored. Given the relatively sound correlation of the WBM prior to this event, this may be attributed to immediate site water management including storage transfers following this event that are not consistent with those within the site WBM.

During late 2022, the site water inventory was heightened due to accumulated rainfall from 2021-2022. At this point in the model, the observed and modelled inventory appear to diverge. This is suspected to be a result of immediate site water management practices undertaken at the time that are inconsistent with assumptions within the site WBM. Since this time, the gap between monitored and modelled inventory has been reduced as a result of annual calibration exercises, and the model appears well fitted against observed data to the end of 2024.



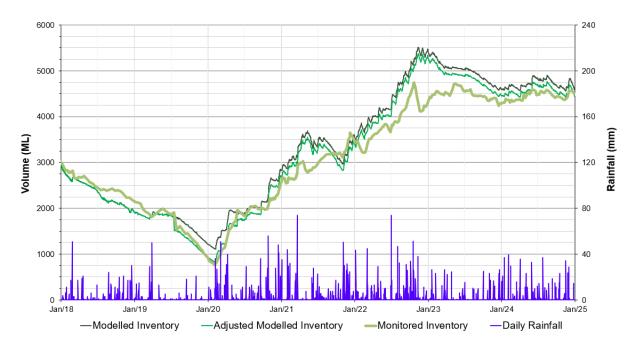


Figure 14: WBM Calibration Simulated vs Measured Combined Site Inventory Key outcomes of the calibration process include:

- Effective representation during significantly dry conditions and during subsequent water recovery;
- Effective representation of inventory reduction through measures such as evaporators and operation of the RO plant; and
- Verification of a series of operating rules regarding groundwater inflow rates, WTF operation and external allow the model to be more effectively used as a predictive tool for on-site water behaviour.

#### 11.4 Salt Balance Verification

The WBM maintains a running account of salt mass in all water storages which is equated to and reported as EC. Model verification of the salt balance has been undertaken using salinity monitoring for the 2024 calibration period (1 January 2018 to 31 December 2024) which only includes storages CWD, RWD and Pit 2W due to the limited availability of monitoring data for this period. The objective of this verification is to establish that salt transfer is effectively being captured within the WBM.

Salt mass inflows are typically estimated by assigning salinity concentrations to runoff from various land use types, and point water sources (e.g., groundwater, pipeline water) as described in **Section 10.0**. Increased salt concentrations are also recirculated into the WMS via the concentrate return from the WTF, directed to the RWD following decommissioning of Pit 1S. No salinity is lost via evaporation from storages.

From available monitoring data, it is found that water circulating through the Water Management System (WMS) is typically within the EC range of 3,000 and 4,000  $\mu$ S/cm. Where data is unavailable, the initial conditions within storages was assumed to be within this range. Model simulated salinity has been compared against historical measurements in **Figure 15** to **Figure 17** for the period January 2018 to December 2024. The results have been plotted for the storages CWD, RWD and Pit 2W.

A key outcome of this verification process is the effective representation of salinity that aligns with trends in monitored data (where available).



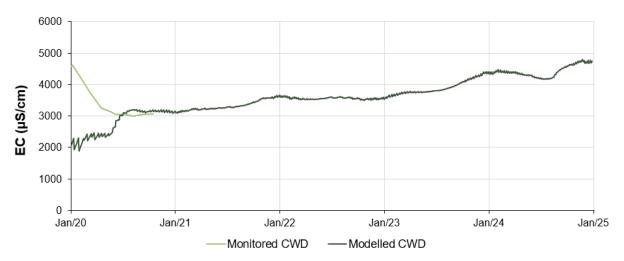


Figure 15: Salinity Verification Simulated vs Measured – CWD

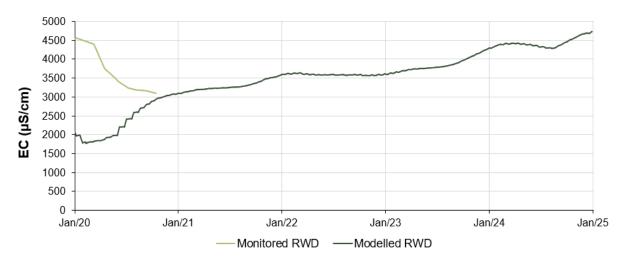


Figure 16: Salinity Verification Simulated vs Measured – RWD

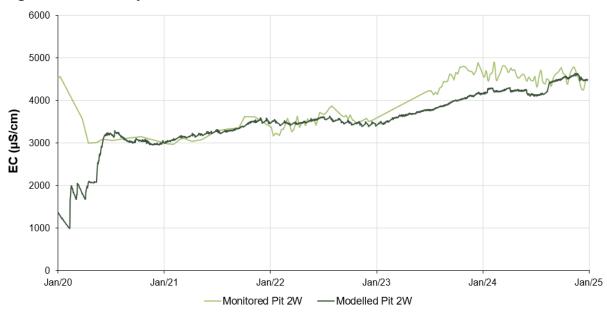


Figure 17: Salinity Verification Simulated vs Measured – Pit 2W



# 11.5 Base Case Model Operating Rules

Representative operating rules that define the Wilpinjong WBM are summarised in **Table 17**. The operating rules have been refined by calibration against monitored data over a seven-year period.

**Table 17: Wilpinjong WBM Operating Rules** 

Item	Description	Operating Rules
1.0	External Water	
1.1	External Water Supply	Water imported from an external source to sustain mine water demands during prolonged drought periods
		External water supplied when site inventory below 2,000 ML, import rate dependent on site inventory level and ranges from 5.1 L/s to 27.3 L/s (see Section 8)
		Inflow directed to CWD
2.0	Supply to Dema	nds
2.1	CHPP	Modelled as a net water extraction of 139 ML/month (4 ML/day) sourced evenly between the CWD and RWD
		Usage consistent with CHPP water balance and forecast production (WRM, 2019) (see Section 5.1.1.4)
		No return from demand
2.2	Miscellaneous Industrial Area	Modelled as a net water extraction of 20 ML/month (0.66 ML/day) sourced evenly between the CWD and RWD
		Assumed loss rate of 0.274 ML/day (100 ML/year)
		Balance assumed to return to Pit 2W
2.3	Dust suppression	Water usage calculated daily in model as a function of climate and application area (Refer to <b>Section 5.2.2</b> )
		No dust suppression if rainfall exceeds 1.5 mm/day
		Demand supplied based on the following breakdown:
		o ROM FP (RWD) – 75.08%
		o Pit 2 FP (Pit 2W) – 24.91%
		o Pit 5 FP (Pit 5 FP Dam) – 0.01%
0.4	F	No return from demand modelled
2.4	Evaporators	Modelled as a net 0.25 ML/day loss from Pit 2W up to 1 July 2024, then as a net 0.375 ML/day loss after 1 July 2024 based on advice from WCM (SLR, 2024b)
		Outflow stream assumed to be water only, no salt removed from Pit 2W
		Disabled if site water inventory is less than 4,000 ML
2.5	WTF	Used to draw down mine water inventory, operated if inventory in WMS exceeds 2,000 ML
		Supplied from Pit 2W at up to 4.0 ML/day, flowrate modelled dependent of previous 5-day rainfall (see <b>Section 6.3</b> )
		Permeate recovery modelled as 75% of feed. No reduction in recovery modelled due to high feed water EC
		Permeate EC modelled at 180 μS/cm
		WTF reject EC modelled as a function of feed water EC based on salt mass balance
		WTF reject pumped to Pit 1S prior to Q4 2018 after which reject pumped to RWD. If Pit 1S/RWD full, reject pumped to Pit 2W. Following recommission in December 2020, reject is pumped to Pit 2W



Item	Description	Operating Rules
		<ul> <li>Discharge water EC modelled at 350 µS/cm, achieved by adding Pit 2W water to the residual permeate stream assumed 0.3 ML/day based on average EC of Pit 2W and discharge water</li> </ul>
3.0	Operation of Ke	y Storages
3.1	Water Storages	
3.1.1	Pit 2W	Primary hub mine water storage
		<ul> <li>Supplies makeup water to the following locations as required:</li> <li>RWD and CWD</li> <li>Pit 2 FP</li> </ul>
		o Pit 5 FP Dam
		Receives pumped dewatering from Pit 5N, Pit 4, Pit 3N, Pit 6, Pit 8 and Pit 8     CWD
		Pumps to Pit 5N at 100 L/s (8.6 ML/day). If Pit 5N is full, Pit 2W pumps to Pit 4 at 100 L/s (8.6 ML/day), and then to Pit 3 as a last resort at 90 L/s (7.8 ML/day).
		Seeps to Pit 4 via Pit 2/4 spoil aquifer
		Supplies water to WTF for treatment and discharge to Wilpinjong Creek under EPL 12425
		Feed water for evaporator spray system
		Exchanges water with adjacent Pit 2/4 spoil aquifer to maintain equalised water levels (exchanges water with Pit 2 half of spoil aquifer only)
		No spillway overflows modelled
3.1.2	RWD	Mine water dam in the CHPP/MIA area
		Supplies water to the following locations:
		<ul> <li>CHPP process water makeup</li> <li>MIA/CHPP miscellaneous water usage</li> <li>ROM FP</li> </ul>
		Sources water from Pit 2W to maintain water level at 412.6 mAHD (295 ML)
		Receives reject from WTF following decommission of Pit 1S in Q4 2018
242	CMD	No spillway overflow modelled
3.1.3	CWD	Mine water dam located north of CHPP/MIA, within the rail loop
		Supplies water to the following locations:
		<ul> <li>CHPP process water makeup</li> <li>MIA/CHPP miscellaneous water usage</li> </ul>
		<ul> <li>MIA/CHPP miscellaneous water usage</li> <li>Sources water from Pit 2W to maintain water level at 395.7 mAHD (30 ML)</li> </ul>
		No spillway overflow modelled
3.1.4	Pit 1S (offline	RO reject storage dam
	as of Q4 2018)	Receives pumped inflow of reject from WTF
		Maximum operating level defined as 421.4 mAHD (295 ML) to minimize seepage to downstream areas within the WMS
		Constant seepage rate of 1 mm/day modelled. Seepage assumed to report to Pit 1/5/6 spoil aquifer
		Additional seepage of 0.45 ML/day to Pit 1/5/6 spoil aquifer modelled if water level exceeds 422.4 mAHD (345 ML)
3.1.5	Pit 5 FP Dam	Water supply for Pit 5 FP
		Receives pumped inflows from Pit 5N and Ed's Lake
		Sources makeup water from Pit 2W to maintain a minimum water level of 391.5 mAHD (3 ML)
		Spillway overflow to Pit 5N at 392.2 mAHD (full storage volume 8.5 ML)



Item	Description	Operating Rules
3.1.6	Ed's Lake	Residual void left within backfilled and rehabilitated Pit 1N void
		Supplies makeup water to Pit 5 FP Dam
		Pumps excess water to Pit 2W at 100 L/s (8.64 ML/day)
		Seepage to underlying Pit 1/5/6 spoil aquifer modelled at 0.5 ML/day
		Spillway overflow to Wilpinjong Creek at 375.3 mAHD (storage capacity nominally 110 ML)
3.1.7	Pit 8 Clean	Constructed in 2020
	Water Dams	Capture water from the Pit 8 upstream diversion
		Excess water pumped to Pit 2W at 160 L/s when volume reaches 6.5 ML prior to 2021, after which water is discharge via LDP 30 at up to 200 L/s
3.2	Tailings Storage	Facilities
3.2.1	All TD's	Old tailings storage cells
		All receive local catchment runoff with no pumped inflows
		No pumped outflows modelled. Standing water left to evaporate, or seep to Pit 2/4 spoil aquifer (at an assumed rate of 2 mm/day)
3.3	Mining Pits	
3.3.1	Pit 5N	Pumps to Pit 5 FP Dam if it requires water. Excess water pumped to Pit 2W at 180 L/s (15.6 ML/day) unless receiving storage is above its maximum operating level
		Maximum water level of 369 mAHD modelled. If water level exceeds this threshold, pumping to Pit 2W will occur regardless of downstream inventory (this will trigger filling of next pit in sequence)
		Receives groundwater inflow of 25% of total inflow prior to 2020, receives 30% groundwater inflow following the commencement of mining Pit 8 (modelled via Pit 1/5/6 spoil aquifer). No groundwater inflow is assumed after 2022
		Exchanges water with adjacent Pit 1/5/6 spoil aquifer to maintain equalised water levels
		Receives seepage from up-dip pits (Pit 5S, Pit 6 and Pit 1) via spoil aquifer
3.3.2	Pit 5S	Seepage to Pit 5N (via Pit 1/5/6 spoil aquifer) modelled as a depth loss rate of 300 mm/day
		No pumped dewatering
3.3.3	Pit 4	Receives seepage from Pit 2W via Pit 2/4 spoil aquifer
		Excess water pumped to Pit 2W at 160 L/s (14.0 ML/day) unless receiving storage is above its maximum operating level
		Maximum water level of 362.0 mAHD modelled. If water level exceeds this threshold, pumping to Pit 2W will occur regardless of downstream inventory (this will trigger filling of next pit in sequence)
		Receives groundwater inflow of 25% of total inflow prior to 2020, receives 20% groundwater inflow following the commencement of mining Pit 8. No groundwater inflow is assumed after 2022
		Exchanges water with adjacent Pit 2/4 spoil aquifer to maintain equalised water levels (exchanges water with Pit 4 half of spoil aquifer only)
3.3.4	Pit 1	Seepage to Pit 1/5/6 spoil aquifer modelled as a depth loss rate of 300 mm/day
		No pumped dewatering
3.3.5	Pit 2S	Seepage to Pit 2/4 spoil aquifer modelled as a depth loss rate of 300 mm/day
		No pumped dewatering



Item	Description	Operating Rules
3.3.6	Pit 3	Receives drainage from Pit 7
		<ul> <li>Excess water pumped to Pit 2W at 90 L/s (7.8 ML/day) unless receiving storage is above its maximum operating level</li> </ul>
		Maximum water level of 358.0 mAHD modelled. If water level exceeds this threshold, pumping to Pit 2W will occur regardless of downstream inventory
		Receives groundwater inflow of 50% of total inflow prior to 2020, receives 20% groundwater inflow following the commencement of mining Pit 8. No groundwater inflow is assumed after 2022
3.3.7	Pit 7	Passively drains to Pit 3
		No pumped dewatering
3.3.8	Pit 6	Seepage to Pit 5N (via Pit 1/5/6 spoil aquifer) modelled as a depth loss rate of 300 mm/day
		Receives groundwater inflow of 50% of total inflow from 2023 with increased pit development
		No pumped dewatering
3.3.9	Pit 8	No pumped dewatering prior to 2020
		Excess water pumped to Pit 2W at 100L/s
		Receives groundwater inflow of 30% of total inflow from 2020, receives 50%
		groundwater inflow from 2023. Does not receive groundwater inflow prior to 2020
3.4	Spoil Aquifers	
3.4.1	Pit 1/5/6 Aquifer	Modelled as two separate cells: Pit 5 spoil aquifer and Pit 1 spoil aquifer
		Pit 5 spoil aquifer equalises with Pit 5N open cut above 351.0 mAHD
		Pit 5 spoil aquifer equalises with Pit 1 spoil aquifer above 354.0 mAHD
3.4.2	Pit 2/4 Aquifer	Modelled as two separate cells: Pit 2 spoil aquifer and Pit 4 spoil aquifer
		Pit 2 spoil aquifer equalises with Pit 2W open cut above 350.75 mAHD
		Pit 4 spoil aquifer equalises with Pit 4 open cut above 331 mAHD
		Pit 2 spoil aquifer seeps to Pit 4 spoil aquifer at a fixed rate of 10 ML/day (seepage calculation based on level difference cannot be modelled within OPSIM due to large head difference – i.e., unstable calculation)
4.0	Other	
4.1	Climate	All water storages receive catchment runoff and lose water to evaporation
4.2	Groundwater	Passive groundwater inflow is experienced due to active mining
	Inflow	Groundwater inflow is determined using adjustment factors to simulate rainfall and recharge responses (see <b>Section 9.1.4</b> )
		Inflow directed to downdip pits within void areas, Pit 5N (pre-2023), Pit 4 (pre-2023), Pit 3 (pre-2023), Pit 6 (post-2023) and Pit 8 (post-2019). The total expected rate is apportioned as follows:
		o Pit 1/5/6 void: 25% (prior to 2020), 30% (from 2020), 50% (from 2023)
		o Pit 2/4 void: 25% (prior to 2020), 20% (from 2020), 0% (from 2023)
		o Pit 3/7 void: 50% (prior to 2020), 20% (from 2020), 0% (from 2023)
		o Pit 8 void: 30% (from 2020), 50% (from 2023)



# 11.6 Performance of Site WMS During Drought Conditions

As discussed in **Section 4.0**, during 2018 and 2019 significant drought conditions were experienced in the region. As a result, water within the site water inventory was seen to decrease to a minimum of 760 ML during this period. In order to preserve site water supplies, a number of strategies have been implemented at WCPL including:

- Operation of the revised CHPP model which includes a BFP (as opposed to direct pumping into tailings dams); and
- The use of DAS to reduce dust generation on roads, hardstand and laydown areas and reduces the need for water carts.

As discussed in **Section 5.1.1**, the CHPP tailings circuit was modified in April 2015 to include a BFP, which dewaters the tailings stream and allows this material to be disposed of as a dry waste stream with the coarse reject. Any moisture bleed-off from within the BFP process is captured and re-circulated to the clarified water tank, thereby reducing the net water usage of the CHPP.

In order to reduce water resources required in times of increased demand for dust suppression, WCPL implemented the use of DAS in 2019. During 2019, minimal rainfall fell at the site (equivalent to a 1st percentile historical rainfall) which increased the need for dust suppression significantly, as illustrated in **Section 5.2**. It is presumed water consumption and associated loss of site water to evaporation would have increased without DAS usage.

Although the site water inventory reduced significantly during this drought period, the above practices along with reduced discharge from the WTF, import of external water sources and effective management of site water storages ensured the site could operate effectively throughout this period.

# 11.7 Performance of Site WMS During Very Wet Conditions

As discussed in **Section 4.0**, during 2020, 2021 and 2022 significant rainfall conditions were experienced in the region. As a result, water within the site water inventory was seen to increase to a maximum of 4723 ML during this period.

To relieve excess water supply, several strategies have been implemented at WCPL including:

- Utilisation of water disposal infrastructure including site WTF and evaporation sprays;
- Discharge of clean water via Pit 8 CWD; and
- EPL licence variation notice to emergency discharge via three LDPs (30, 31, 32).

As discussed in **Section 7.2**, emergency discharge was undertaken from October 2022 to January 2023. During the period 2020 to 2022 above 90<sup>th</sup> percentile annual rainfall was experienced in all years including 97<sup>th</sup> percentile annual rainfall in 2022. Consecutive wet climatic conditions significantly increased the need for water reduction. Effective site management measures ensured the site could operate effectively throughout this period.



# 12.0 Forecast of Site Water Behaviour

The Wilpinjong WBM as described in this report has been utilised to investigate the behaviour of the site water inventory for the 3-year forecast period from 1 January 2025 to 31 December 2027.

# 12.1 Model Configuration

The WBM has been configured to account for changes required to simulate site operations scheduled to occur in future. The WBM primarily operates as per the configuration described in this report, however, adjustments have been made to the simulation methodology, catchment breakdown, CHPP Demand, site WMS operations. These elements are described in the following sections.

## 12.1.1 Simulation Methodology

The WBM was run on a daily timestep for the period between 1 January 2025 and 31 December 2027. As described in **Section 4.2** and **4.3**, 125 years of climate data sourced from the SILO Data Drill is available for WCM for use in analysis in long-term climate trends. Stochastic climate data has been used to determine rainfall patterns for the forecasted years.

The purpose of stochastic rainfall generation is to develop a wide range of climate sequences based on the recorded rainfall data of the area. These sequences have comparable statistical characteristics to that of the historical dataset for a range of parameters, including mean, variance, skew, and count of wet or dry days. Each sequence has an order in which the rainfall has occurred. For example, one sequence may have wetter years at the start of the sequence, where another sequence may have the wetter years towards the end of the sequence. Some sequences may be wetter or dryer than others in order to account for the variability of the climate which may occur during the LOM. The probabilistic rainfall data replicates the seasonality of the historical rainfall data.

The probabilistic climate data for the WBM was used to predict rainfall at the site during the forecast period to determine the volume of water on site which needs to be managed. The probabilistic rainfall sequences were produced through the use of the Stochastic Climate Library (SCL) software (eWater CRC).

Stochastic rainfall data was produced for 500 replicates of 3-year rainfall data (1,500 years of probabilistic data total). This allows a broad range of climatic conditions to be simulated, which then gives the mean and median of the assessment. The assessment also yields percentiles which are interpreted as a percentage exceedance probability (i.e., the likelihood of an event occurring).

Monthly evaporation rates have been utilised for the forecast period as per **Section 4.3**. Due to limitations in the number of stations in the region, long-term average values were used in the WBM as opposed to stochastic evaporation data.

The stored volumes prior to the simulated forecast period (to 31 December 2024) were estimated based on monitored water level data recorded by WCPL. The combined site volume to the latest date prior to the prediction period was 4,545 ML on 24 December 2024.



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The results of the site water inventory are presented in terms of the following climatic conditions:

- Very Wet Climatic 99th percentile results of the volume predicted using the 500 probabilistic climatic sequences:
- Wet Climatic 90th percentile results of the volume predicted using the 500 probabilistic climatic sequences:
- Median Climatic 50th percentile results of the volume predicted using the 500 probabilistic climatic sequences;
- Dry Climatic 10th percentile results of the volume predicted using the 500 probabilistic climatic sequences; and
- Very Dry Climatic 1st percentile results of the volume predicted using the 500 probabilistic climatic sequences.

#### 12.1.2 Catchment Breakdown

Catchment boundaries for water storages within WCM along with land use classifications for the years 2025, 2026 and 2027 have been delineated based on the most recent available catchment areas and land use types provided by WCPL and the long-term mine forecast. A breakdown of land use type per water storage catchment area, together with catchment and land use maps, is provided in Appendix B.

#### 12.1.3 **Site Water Management System Operations**

The operations within the site water management system for the forecast period are expected to be generally consistent with the arrangement described throughout this report. Catchment areas for the forecast period are shown in **Appendix B**.

Evaporator sprays were recommissioned in 2022 and therefore are allowed to draw water from Pit 2W in accordance with threshold rules described in **Section 5.3**.

#### 12.1.4 CHPP Demand

As the model has been updated to include a time series of CHPP water demand based on the monitored usage, for model forecasting an annual average across the calibration period of 3.6 ML/day has been adopted.

#### 12.2 Outcomes

#### 12.2.1 Water Balance

Model simulated volumes have been forecast for the period 1 January 2025 to 31 December 2027. Results have been plotted for the combined water inventory in the WMS (comprising CWD, Pit 1S, Pit 2W, Pit 3, Pit 4, Pit 5N and the RWD).

Figure 18 shows the forecasted total site inventory and associated WTF discharge for the period 1 January 2025 to 31 December 2027 through varying climatic conditions.



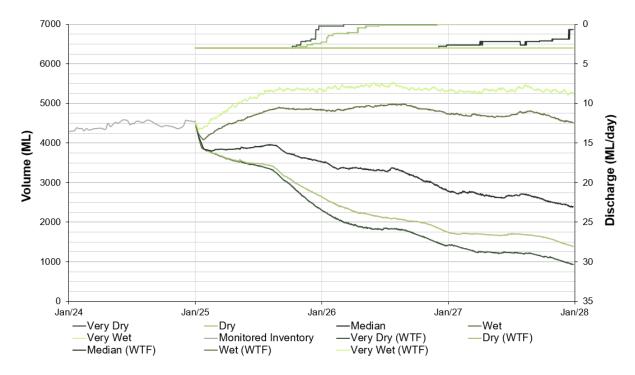


Figure 18: Forecast Site Water Inventory – 2025 to 2027

Review of Figure 18 shows the following:

- The 1<sup>st</sup> percentile (very dry climatic conditions) results in a total site water decrease to 2,315 ML at the end of 2025, 1,413 ML at the end of 2026, and 934 ML at the end of 2027;
- The 10<sup>th</sup> percentile (dry climatic conditions) results in a total site water decrease to 2,655 ML at the end of 2025, 1,753 ML at the end of 2026, and 1,387 ML at the end of 2027;
- The 50<sup>th</sup> percentile (median climatic conditions) results in a total site water decrease to 3,535 ML at the end of 2025, 2,802 ML at the end of 2026, and 2,396 ML at the end of 2027;
- The 90<sup>th</sup> percentile (wet climatic conditions) results in a total site water decrease to 4,847 ML at the end of 2025, 4,736 ML at the end of 2026, and 4,510 ML at the end of 2027; and
- The 99<sup>th</sup> percentile (very wet climatic conditions) results in a total site water decrease to 5,355 ML at the end of 2025, 5,318 ML at the end of 2026, and 5,243 ML at the end of 2027.

Overall, the forecast indicates that there is adequate water security during dry conditions, with opportunities to reduce inventory by destruction. Water inventory during very wet years will be manageable, however, the site will need to remain proactive and implement strategies to reduce inventory as implemented following the 2022 wet period. It is understood that WCM are currently investigating the feasibility of increasing discharge rates from the WTF to Wilpinjong Creek to relieve surplus inventory during 2025.



#### 12.2.2 Salt Balance

Model simulated salinity has been forecast for the period 1 January 2025 to 31 December 2027. Results have been plotted for the primary transfer storages within the combined water inventory in the WMS (i.e., CWD, RWD, and Pit 2W).

Salinity has been presented in terms of salinity percentile of salinity levels that may result from the varying climatic conditions simulated and provides an indication of the range of salinities that may be experienced within storages. Hence, the 99th percentile salinity is the highest 99 percent of possible salinity levels occurring in the water storage and therefore does not necessarily correlate to very wet (99th percentile) rainfall.

Where water within storages becomes significantly low, such as during very dry or dry conditions described in **Section 12.2.1**, the model does not capture all of the processes associated with the movement and transfer of salt. For this reason, the salt concentration of the site water storages has been capped at a maximum of 25,000 mg/L (EC of  $37,313~\mu\text{S/cm}$ ). This rarely activates in the model and typically only applies to very dry or dry climate conditions when storages dry out or reach very low water levels. In these instances, the mass of salt predicted is small but as the volume of water modelled is also small this is reported as a very high salt concentration. In reality, a proportion of the salts would be lost to seepage or settle as sediment in the storage.

Figure 19 shows the forecasted salinity of the CWD throughout the forecast period.

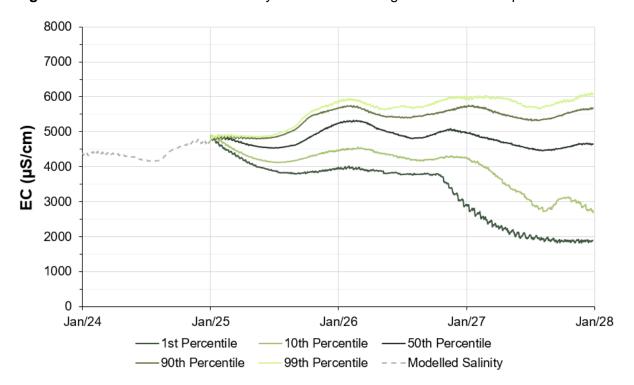


Figure 19: CWD Forecast Salinity – 2025 to 2027

Review of **Figure 19** shows that median salinity within the CWD fluctuates between  $4,400 \mu S/cm$  to  $5,400 \mu S/cm$  throughout the simulation.

Figure 20 shows the forecasted salinity of the RWD throughout the forecast period.



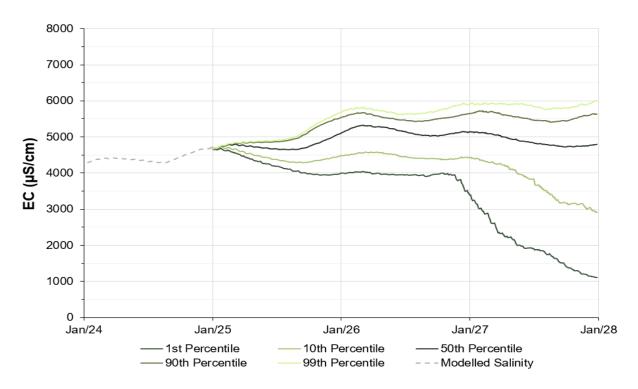


Figure 20: RWD Forecast Salinity – 2025 to 2027

**Figure 20** shows that median salinity within the RWD fluctuates between 4,600  $\mu$ S/cm to 5,400  $\mu$ S/cm throughout the simulation.

Figure 21 shows the forecasted salinity for Pit 2W throughout the forecast period.

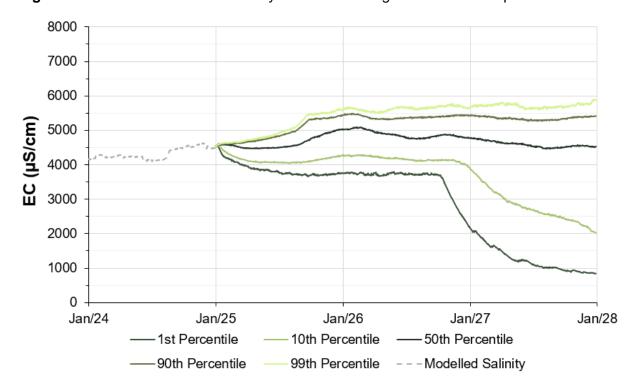


Figure 21: Pit 2W Forecast Salinity – 2025 to 2027

**Figure 21** shows that median salinity within Pit 2W ranges between 4,400  $\mu$ S/cm to 5,250  $\mu$ S/cm throughout the simulation.



#### 13.0 Conclusion and Recommendations

The current investigation has updated the WCM WBM to reflect changes in the WMS and additional monitoring data recorded during 2024. Key outcomes of current investigations include:

- Updated catchment schedule and land use classifications based on information current as at the end of year 2024;
- Overall, the WBM provides a good correlation between monitored and predicted water inventory and provides a sound platform for future studies;
- The salt balance incorporated within the WBM effectively tracks overall salt mass within the site storages; and
- Forecast site mine water inventory behaviour for the period 2025 to 2027 under different site operating scenarios and climatic conditions. Overall, the forecast indicates that the site water inventory is likely to be able to be reduced unless rainfalls exceeding the 90<sup>th</sup> percentile occurs; that water inventory is manageable during very wet years; and that there is adequate water security during dry conditions.

It is recommended that WCM implement improved monitoring of groundwater inflows which will allow for improved calibration on this aspect of the WBM in future studies. Groundwater inflow has previously been inferred by reviewing monitored volumes of water extracted from the WCM open cut pits. It is recommended that pumping records are again reviewed to measure/estimate groundwater inflow. This will enable validation and improved calibration of the WBM and numerical groundwater model in future studies. Specifically, Pit 8 could be a good indicator for groundwater inflows as it has minimal interaction with spoil aquifers and pumped inflows.

The updated WBM is considered to be well suited for planning studies, infrastructure sizing and operational decision making, provided these studies incorporate sensitivity analysis (as any robust study should).

It should be noted that the content of this report may be subject to revision with any future improved understanding of the operational and response characteristics of the WCM water management system.

#### 13.1 Model Limitations

Climatic data (rainfall and evaporation), supply, demand, and transfer volumes have been modelled as daily totals. The model assumes that daily data can be distributed over 24 hours. The model does not accurately represent events with durations less than 24 hours. For example, storm runoff events with durations less than 24 hours cannot be accounted for using the WBM.

The WBM has been developed and calibrated with a focus on the water management system as a whole. Model accuracy is considered better for design applications of wider scope (e.g., site water balance) relative to studies of narrower focus (e.g., single dams). Although the model is well suited for undertaking smaller studies, inputs and controls should always be first understood and then modelled to a level of detail suitable to the task at hand.



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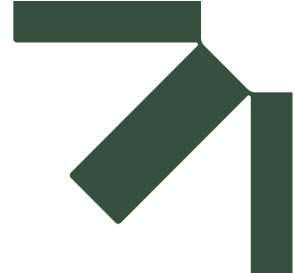
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27 March 2025 SLR Project No.: 630.032087.00001 SLR Ref No.: 630.032087-R01-v1.0-20250327

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# Appendix A Model Schematic

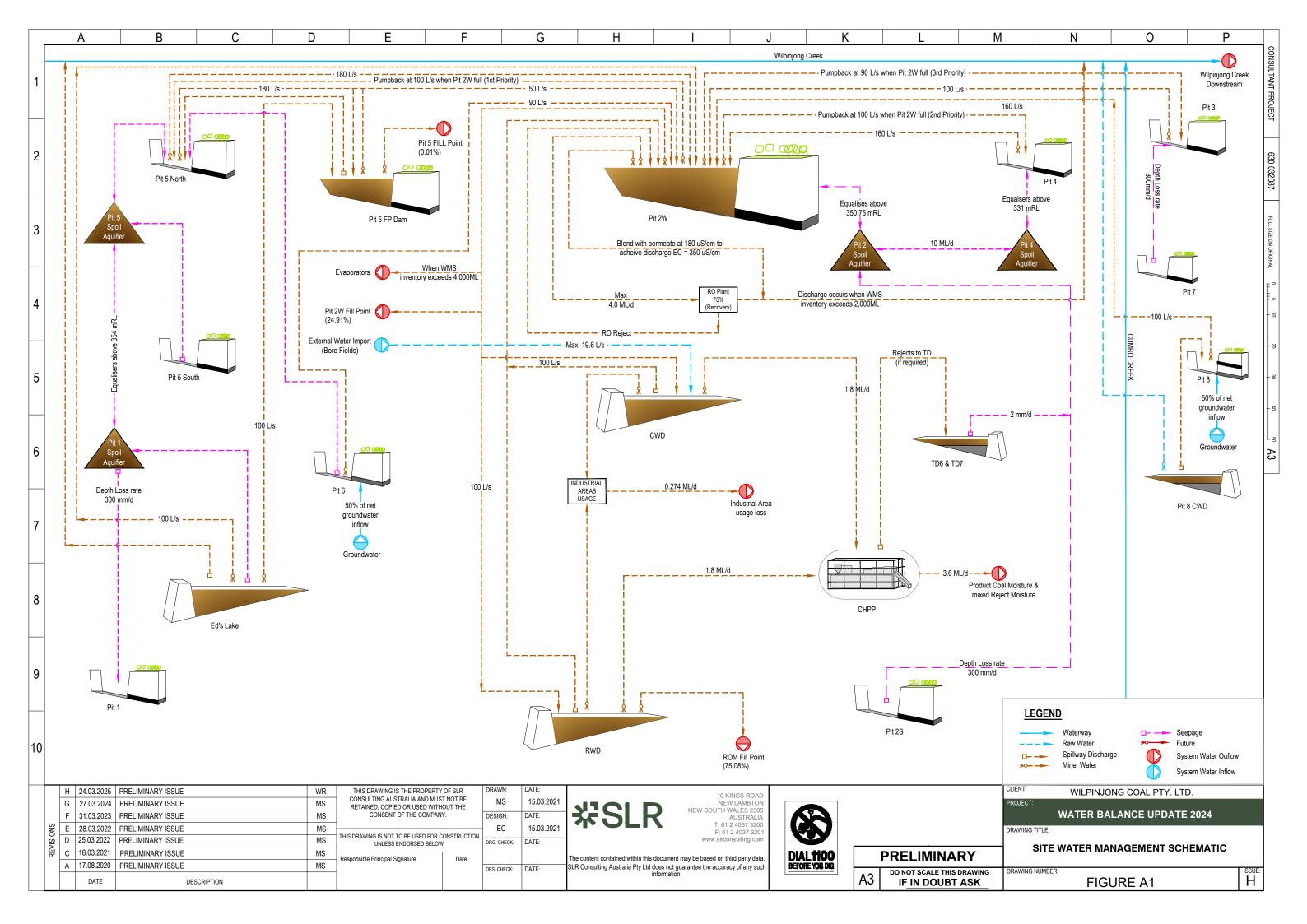
**Water Balance Model Update 2025** 

**Wilpinjong Coal Mine** 

Wilpinjong Coal Pty Ltd

SLR Project No.: 630.032087.00001







# Appendix B Catchments and Land Use

**Water Balance Model Update 2025** 

**Wilpinjong Coal Mine** 

Wilpinjong Coal Pty Ltd

SLR Project No.: 630.032087.00001



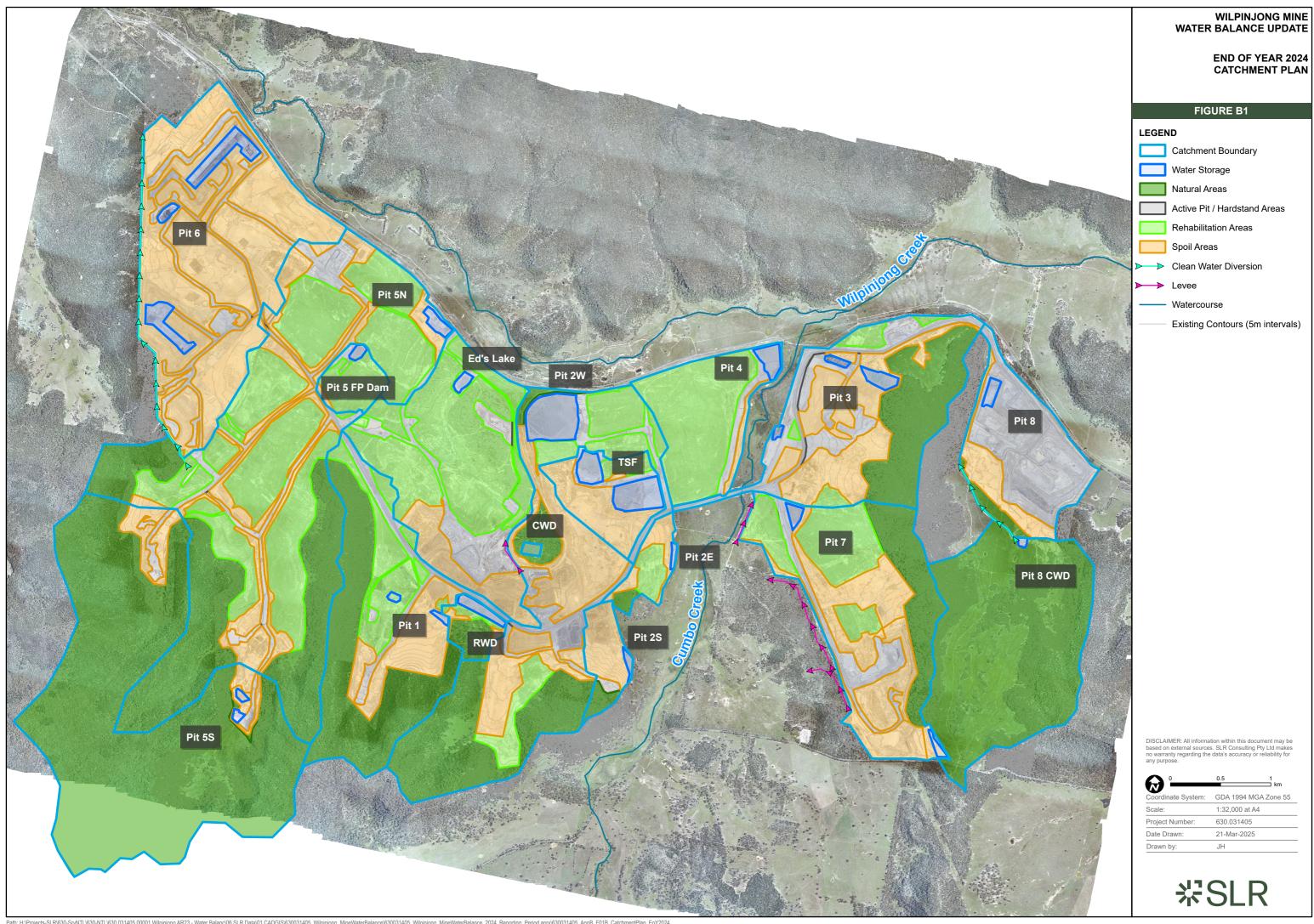


Table B1: 2024 Catchment and Land Type Areas (Based on 2024 End of Year Conditions)

`		Tor rear conditions	,		
Name	Natural (ha)	Rehabilitation (ha)	Spoil (ha)	Hardstand/ Active Pit (ha)	Total (ha)
Water Storages					
Pit 2 West (Pit 2W)	149.4	117.5	76.7	412.9	
Clean Water Dam (CWD)	-	-	-	2.1	2.1
Ed's Lake	-	184.4	43.8	64.1	292.7
Pit 1S	-	-	-	-	-
Pit 5 FP Dam	-	27.4	1.3	4.5	33.2
Recycled Water Dam (RWD)	14.5	3.8	3.0	5.4	26.7
Pit 8 Mine Water Dams (CWD)	306.0	-	-	0.8	306.8
Sediment Dams					
Included in respecti	ve pit catchme	nts.			
Mining Pits					
Pit 1	151.9	58.3	58.0	29.5	297.7
Pit 2 East	4.4	13.3	14.1	1.3	33.1
Pit 2 South	6.6	-	27.3	4.0	38.0
Pit 3	98.8	16.5	96.0	87.5	298.8
Pit 4	-	107.6	6.1	18.6	132.3
Pit 5 North	235.5	329.0	102.8	65.0	732.3
Pit 5 South	550.9	-	31.0	10.8	592.7
Pit 6	-	0.5	328.4	82.9	411.8
Pit 7	64.2	80.4	112.5	43.7	300.8
Pit 8	9.0	-	28.1	112.4	149.5
Other					
Combined (6 & 7) Tailings Dams	-	8.8	42.6	27.3	78.7



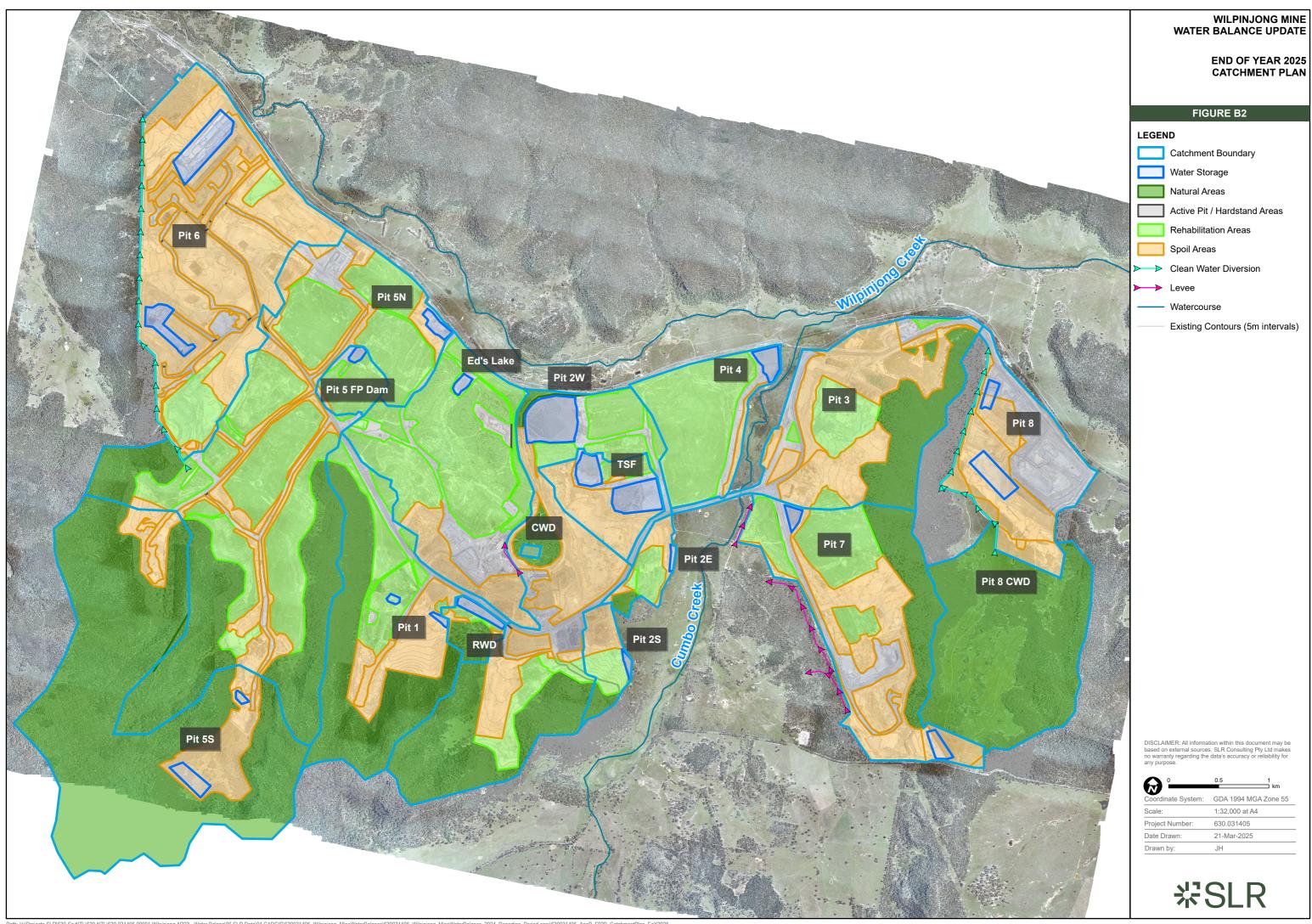


Table B2: 2025 Catchment and Land Type Areas

Name	Natural (ha)	Rehabilitation (ha)	Spoil (ha)	Hardstand/ Active Pit (ha)	Total (ha)
Water Storages					
Pit 2 West (Pit 2W)	149.4	77.0	76.7	412.9	
Clean Water Dam (CWD)	-	-	-	2.1	2.1
Ed's Lake	-	184.4	43.8	64.1	292.7
Pit 1S	-	-	-	-	-
Pit 5 FP Dam	-	27.4	1.3	4.5	33.2
Recycled Water Dam (RWD)	14.5	3.8	3.0	5.4	26.7
Pit 8 Mine Water Dams (CWD)	273.3	-	-	-	273.3
Sediment Dams					
Included in respecti	ve pit catchme	nts.			
Mining Pits					
Pit 1	151.9	58.3	61.1	26.4	297.7
Pit 2 East	4.4	14.8	12.6	1.3	33.1
Pit 2 South	6.6	12.2	15.2	4.0	38.0
Pit 3	80.5	38.8	133.5	46.0	298.8
Pit 4	-	107.6	6.1	18.6	132.3
Pit 5 North	235.5	335.7	98.8	62.3	732.3
Pit 5 South	509.6	-	74.0	9.1	592.7
Pit 6	-	38.6	344.9	50.7	434.2
Pit 7	64.2	80.4	123.5	45.3	313.4
Pit 8	-	-	83.6	106.4	190.0
Other					
Combined (6 & 7) Tailings Dams	-	8.8	42.6	27.3	78.7



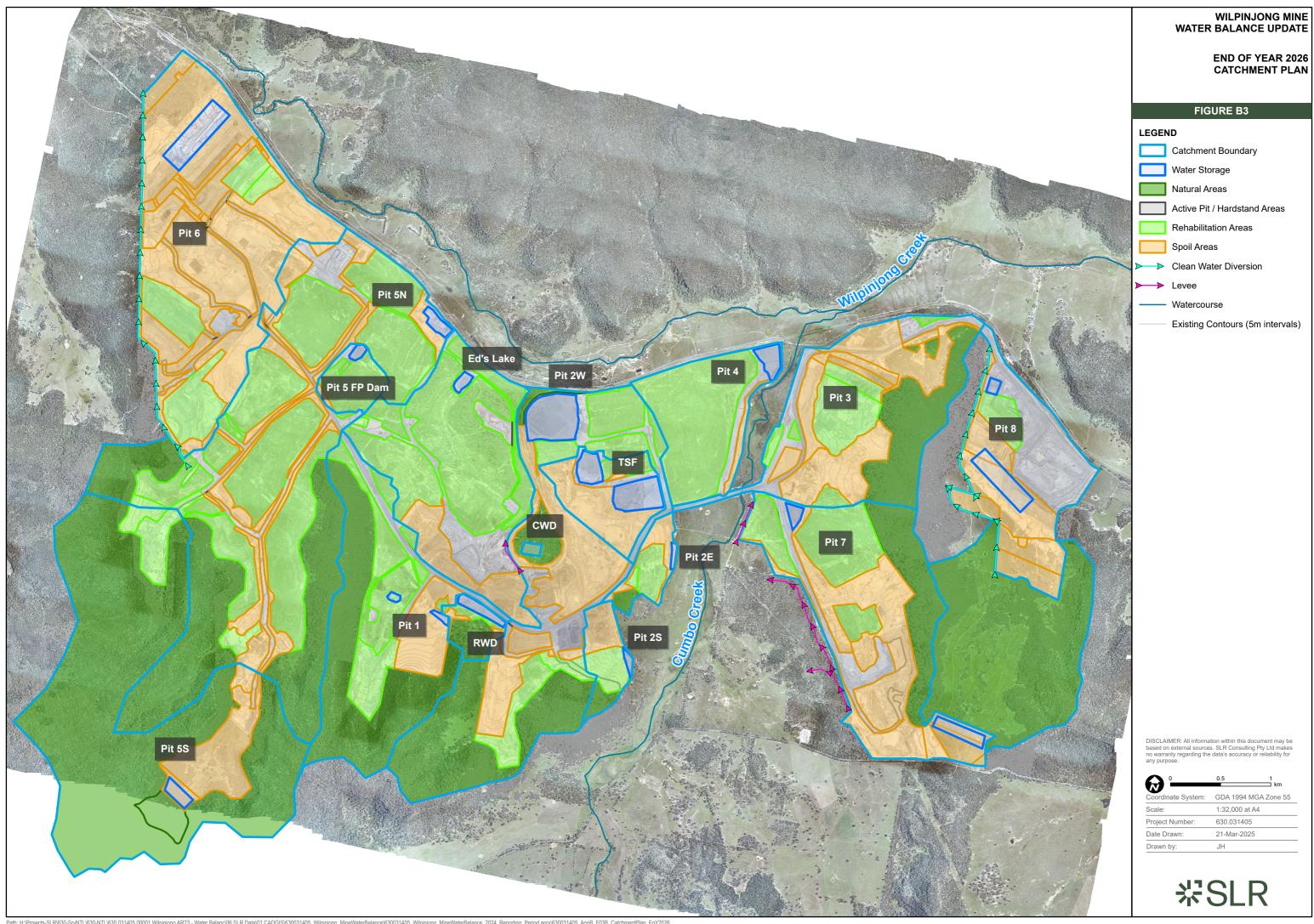


Table B3: 2026 Catchment and Land Type Areas

Name	Natural (ha)	Rehabilitation (ha)	Spoil (ha)	Hardstand/ Active Pit (ha)	Total (ha)
Water Storages				Active i it (iia)	
Pit 2 West (Pit 2W)	149.4	77.0	109.8	76.7	412.9
Clean Water Dam (CWD)	-	-	-	2.1	2.1
Ed's Lake	-	184.4	43.8	64.1	292.7
Pit 1S	-	-	-	-	-
Pit 5 FP Dam	-	27.4	1.3	4.5	33.2
Recycled Water Dam (RWD)	14.5	3.8	3.0	5.4	26.7
Pit 8 Mine Water Dams (CWD)	251.0	-	-	-	251.0
Sediment Dams					
Included in respecti	ve pit catchme	nts.			
Mining Pits					
Pit 1	151.9	81.9	58.0	5.9	297.7
Pit 2 East	4.4	14.8	12.6	1.3	33.1
Pit 2 South	6.6	12.2	15.2	4.0	38.0
Pit 3	80.5	50.0	122.3	46.0	298.8
Pit 4	-	107.6	6.1	18.6	132.3
Pit 5 North	235.5	341.4	95.0	60.4	732.3
Pit 5 South	502.9	25.7	58.8	5.3	592.7
Pit 6	-	71.5	325.9	47.3	444.7
Pit 7	64.2	80.4	126.5	49.3	320.4
Pit 8	-	15.4	103.6	102.1	221.1
Other					
Combined (6 & 7) Tailings Dams	-	8.8	42.6	27.3	78.7



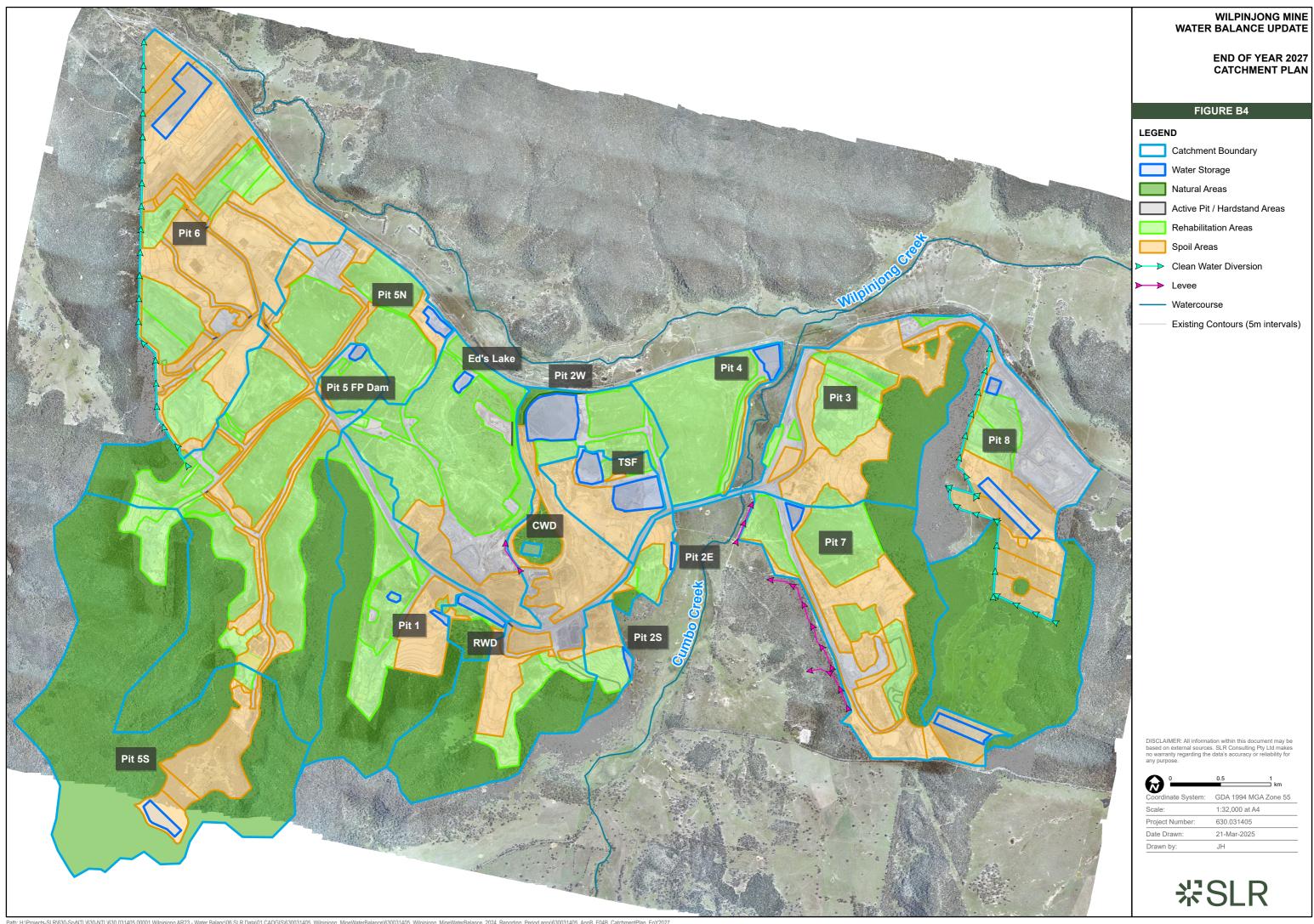


Table B4: 2027 Catchment and Land Type Areas

Name	Natural (ha)	Rehabilitation (ha)	Spoil (ha)	Hardstand/ Active Pit (ha)	Total (ha)
Water Storages					
Pit 2 West (Pit 2W)	149.4	77.0	76.7	412.9	
Clean Water Dam (CWD)	-	-	-	2.1	2.1
Ed's Lake	-	184.4	43.8	64.1	292.7
Pit 1S	-	-	-	-	-
Pit 5 FP Dam	-	27.4	1.3	4.5	33.2
Recycled Water Dam (RWD)	14.5	3.8	3.0	5.4	26.7
Pit 8 Mine Water Dams (CWD)	232.5	-	-	-	232.5
Sediment Dams					
Included in respecti	ve pit catchme	nts.			
Mining Pits					
Pit 1	151.9	81.9	58.0	5.9	297.7
Pit 2 East	4.4	14.8	12.6	1.3	33.1
Pit 2 South	6.6	•	27.3	4.0	38.0
Pit 3	80.5	62.1	110.2	46.0	298.8
Pit 4	-	115.8	3.4	13.1	132.3
Pit 5 North	235.5	344.5	91.9	60.4	732.3
Pit 5 South	488.3	29.2	69.8	5.4	592.7
Pit 6	=	113.6	302.0	42.4	458.0
Pit 7	64.2	98.8	123.0	38.8	324.8
Pit 8	8.2	29.9	85.9	105.6	229.6
Other					
Combined (6 & 7) Tailings Dams	-	8.8	42.6	27.3	78.7





# Appendix C Long-Term Water Quality Data

**Water Balance Model Update 2025** 

**Wilpinjong Coal Mine** 

Wilpinjong Coal Pty Ltd

SLR Project No.: 630.032087.00001



27 March 2025 SLR Project No.: 630.032087.00001 SLR Ref No.: 630.032087-R01-v1.0-20250327

		Monthly Dainfall			Da	ms				Pi	ts				WTF		Ref	Reference (Waterways)		
Year	Month	Monthly Rainfall (mm)	Pit 2W	Pit 1S	Pit 5 FP	CWD	RWD	Ed's Lake	Pit 5	Pit 2 NB	Pit 4	Pit 3	Feed	Permeate	Discharge	Concentrate	Wilp. Ck Upstream	Wilp. Ck Downstream	Cumbo Creek	
2015	Jan	115.7													325					
	Feb	17.5								5,290										
	Mar	16.3	3,048							4,790					310					
	Apr	109.3	3,390	6,670		3,330	3,510	880	1060	4,940	3,960				285					
	May	43.2													210					
	Jun	45.8		9,180						4,100					221					
	Jul	38.4								4,620					144					
	Aug	51.5													185		739	530	5,112	
	Sep	10.6	3,490	5,690	2,110	3,440	3,580		2,290		4,250	3,030			158		1,296	365	5,203	
	Oct	46.9			3,540					5,190					176		1,957	379	6,005	
	Nov	90.3													212		1,007	352	4,694	
	Dec	105.1								4,290					269		883	446		
2016	Jan	99.9	3,280	5,770		3,470	3,440	2,210	2,330	4,940	3,640				267		1,053	431		
	Feb	9.1													255		1,351	441		
	Mar	19.2													235			590		
	Apr	4.4													232					
	May	67.9													195				3,620	
	Jun	107.7		7,700											176			386	6,254	
	Jul	83													208		497	1,082	3,987	
	Aug	43.3													201		792	562	5,582	
	Sep	172	3,310	6,180		3,280	3,320		2,740		3,880				199		313	73	1,942	
	Oct	71.3													235		430	1,100	2,530	
	Nov	44.9													284		536	976		
	Dec	35.6													276		1,446	465		
2017	Jan	34.4	3,545												294			486		
	Feb	25.8	3,520												305	14,000		539		
	Mar	130.4	3,670												301	13,400		686		
	Apr	19.4	3,620												307			539	1,431	
	May	23.4	3,660												276			359	4,804	
	Jun	11.8	3,630												347			344	5,796	
	Jul	1.9	3,580												372			272	5,716	
	Aug	26.4													357			285	5,365	
	Sep	76.3													336			26	5,745	
	Oct	33.3	3,710	3,710			7,610								321			290	6,280	
	Nov	76.3	3,950	3,950											335			310		
	Dec	82.3													342			384		
2018	Jan	15.7															4,110	599		
	Feb	60.7																1,500	476	



					Da	ms				Pit	ts		WTF				Reference (Waterways)		
Year	Month	Monthly Rainfall (mm)	Pit 2W	Pit 1S	Pit 5 FP	CWD	RWD	Ed's Lake	Pit 5	Pit 2 NB	Pit 4	Pit 3	Feed	Permeate	Discharge	Concentrate	Wilp. Ck Upstream	Wilp. Ck Downstream	Cumbo Creek
	Mar	45.2															4,360	2,020	3,690
	Apr	37.4															2,363	590	237
	May	13.4															2,147	424	6,950
	Jun	24.2															1,805	351	3,776
	Jul	7.5													288		1,726	375	6,820
	Aug	29													312		1,656	356	3,655
	Sep	48.9													229		1,600	385	3,521
	Oct	51.3													328		1,781	418	3,629
	Nov	49.6													365		2,001	437	3,977
	Dec	105													367				
2019	Jan	82.3	4,350																
	Feb	4.8	4,290																
	Mar	107.3	4,340																
	Apr	0	4,250																
	May	18.9	4,170																
	Jun	7.2	4,010																7,860
	Jul	3.2	4,120																7,077
	Aug	7.5	4,120			4,100	3,990												6,956
	Sep	25.1	4,260			4,180	4,250												7,580
	Oct	5.6	4,400																
	Nov	26.2				4,350	4,370												
	Dec	4.2	4,430																
2020	Jan	27	4,550			4,610	4,550												
	Feb	137																1,190	4,940
	Mar	92	3,560			3,740	4,390											2,650	4,025
	Apr	117	2,990			3,260	3,750										532	510	5,850
	May	16	3,000			3,140	3,530										660	744	6,270
	Jun	23	3,080			3,060	3,410										698	835	5,575
	Jul	70	3,050			3,050	3,240										467	545	5,500
	Aug	36	3,080			3,000	3,190										260	311	4,330
	Sep	77	3,110			3,060	3,170										291	420	3,907
	Oct	151	3,140			3,070	3,100										518	492	7,120
	Nov	17															458	464	
	Dec	162															471	629	7,050
2021	Jan	53	2,970												367				
	Feb	127	3,020												390				7,220
	Mar	160													352				7,870
	Apr	2	3,100												345				6,880



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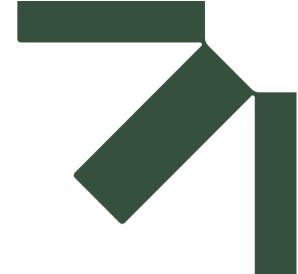
		Monthly Dainfall			Da	ıms				Pi	ts				WTF		Reference (Waterways)		
Year	Month	Monthly Rainfall (mm)	Pit 2W	Pit 1S	Pit 5 FP	CWD	RWD	Ed's Lake	Pit 5	Pit 2 NB	Pit 4	Pit 3	Feed	Permeate	Discharge	Concentrate	Wilp. Ck Upstream	Wilp. Ck Downstream	Cumbo Creek
	Мау	9	3,050												381				6,700
	Jun	84													361				3,335
	Jul	67	3,280												371				3,115
	Aug	25	3,320										3,317		382				
	Sep	44	3,350										3,472		375				
	Oct	31	3,610										3,683		398				
	Nov	249	3,610										3,685		395				
	Dec	81											3,334		366				
2022	Jan	101											3,238				294	812	3,143
	Feb	16											3,310				432	540	3,655
	Mar	120											3,393				317	901	2,758
	Apr	95											3,483				352	1,411	2,842
	May	44											3,502				311	1,588	2,781
	Jun	13											3,693				337	1,516	2,645
	Jul	136											3,738				198	1,136	1,763
	Aug	103											3,860				175	1,026	1,374
	Sep	94											3,610				172	1,014	1,753
	Oct	185											3,535				145	754	1,170
	Nov	64											3,570				492	1,944	1,207
	Dec	27											3,513					1,787	1,885
2023	Jan	49											3,562		406		1,755	1,673	2,188
	Feb	25											3,853		411		791	1,145	2,409
	Mar	65											3,883		412		1,040	1,098	3,056
	Apr	48											3,915		405		787	1,311	3,087
	May	3											3,916		393		728	1,352	2,965
	Jun	29											4,023		388		586	1,370	3,008
	Jul	23	4,188												376		568	1,312	2,875
	Aug	30	4,322												378		708	1,234	3,037
	Sep	18	4,533												382		935	1,057	3,255
	Oct	36	4,724												374		1,209	804	3,777
	Nov	94	4,705												375		1,338	936	3,532
	Dec	59	4,785												378		1,431	703	3,867
2024	Jan	87	4,610												386		881	714	3,938
	Feb	79	4,570												369		793	614	5,336
	Mar	33	4,730												393		1,118	564	5,418
	Apr	69	4,570												393		1,001	623	ND
	May	62	4,560												407		919	660	4,384
	Jun	69	4,460												383		654	810	3,802



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		Monthly Rainfall			Da	ms				Pit	s				WTF		Reference (Waterways)			
Year	Month			Pit 1S	Pit 5 FP	CWD	RWD	Ed's Lake	Pit 5	Pit 2 NB	Pit 4	Pit 3	Feed	Permeate	Discharge	Concentrate	Wilp. Ck Upstream	Wilp. Ck Downstream	Cumbo Creek	
	Jul	65	4,440												407		683	953	3,519	
	Aug	40	4,330												428		633	1,181	3,426	
	Sep	45	4,690											37	424		731	771	3,564	
	Oct	51	4,570												405		716	775	3,744	
	Nov	117	4,690												382		724	572	4,672	
	Dec	45	4,260												442		558	680	3,608	





# **Appendix D** Storage Curves

Water Balance Model Update 2025

**Wilpinjong Coal Mine** 

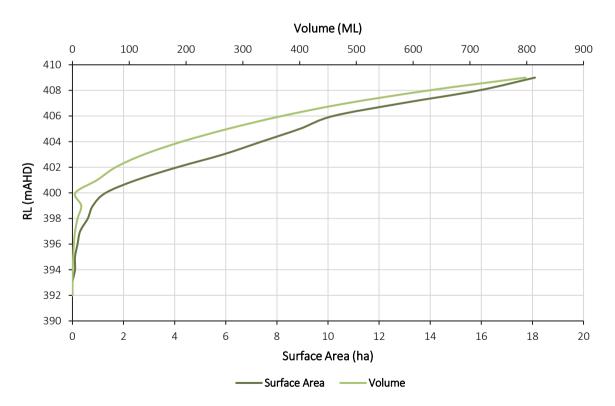
Wilpinjong Coal Pty Ltd

SLR Project No.: 630.032087.00001



Pit 1

RL (mAHD)	Area (ha)	Volume (ML)
392	0	0
393	0	0
394	0.1	0.4
395	0.1	1.3
396	0.2	2.5
397	0.3	4.6
398	0.6	9
399	0.8	15.5
400	1.3	5.1
401	2.5	43.4
402	4.1	76
403	5.9	126
404	7.4	192.7
405	8.9	274
406	10.2	368.9
407	12.9	483.4
408	15.9	627.6
409	18.1	797.8



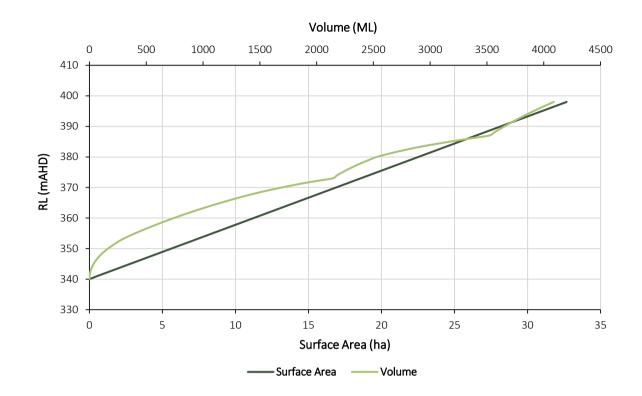


# Pit 2W

RL (mAHD)	Area (ha)	Volume (ML)
340	0	0
341	0.5	1
342	1.1	5
343	1.7	12
344	2.2	23
345	2.8	37
346	3.4	54
347	3.9	75
348	4.5	101
349	5.0	130
350	5.6	164
351	6.2	201
352	6.7	242
353	7.3	288
354	7.9	343
355	8.4	403
356	9.0	466
357	9.6	532
358	10.1	600
359	10.7	671
360	11.3	744
361	11.8	820
362	12.4	899
363	12.9	981
364	13.5	1067
365	14.1	1157
366	14.6	1251
367	15.2	1349
368	15.8	1453
369	16.3	1567

RL (mAHD)	Area (ha)	Volume (ML)
370	16.9	1692
371	17.5	1828
372	18.0	1977
373	18.6	2142
374	19.1	2181
375	19.7	2228
376	20.3	2279
377	20.8	2335
378	21.4	2395
379	22.0	2459
380	22.5	2529
381	23.1	2630
382	23.7	2744
383	24.2	2867
384	24.8	3012
385	25.3	3173
386	25.9	3342
387	26.5	3519
388	27.0	3563
389	27.6	3607
390	28.2	3654
391	28.7	3701
392	29.3	3751
393	29.9	3802
394	30.4	3855
395	31.0	3910
396	31.5	3967
397	32.1	4026
398	32.7	4088



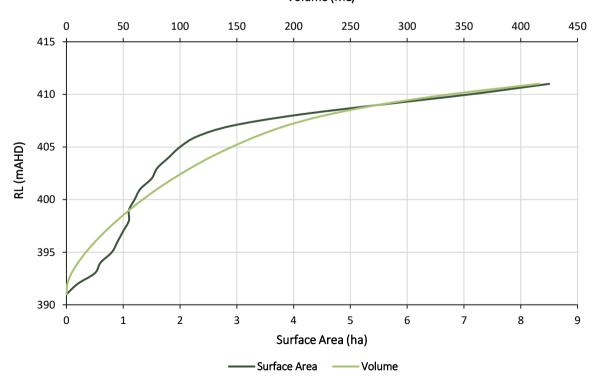




# Pit 2 South

RL (mAHD)	Area (ha)	Volume (ML)
391	0	0
392	0.2	0.8
393	0.5	4.5
394	0.6	10.2
395	0.8	17.2
396	0.9	25.3
397	1	34.4
398	1.1	44.4
399	1.1	55.3
400	1.2	67.1
401	1.3	79.9
402	1.5	94
403	1.6	109.4
404	1.8	126.3
405	2	145.5
406	2.3	167.1
407	2.9	192.8
408	4	227.2
409	5.5	274.4
410	7.1	337.3
411	8.5	415.8

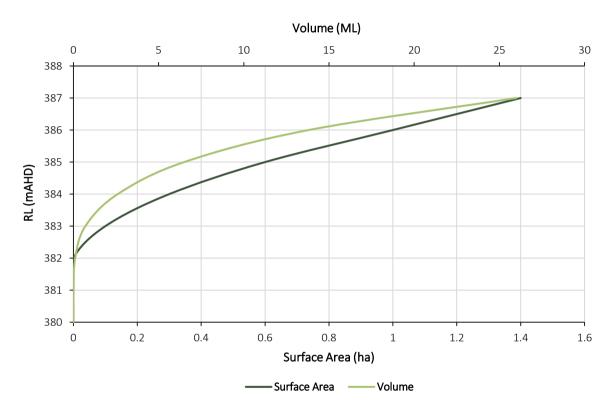






# Pit 2 East

RL (mAHD)	Area (ha)	Volume (ML)
380	0	0
381	0	0.01
382	0	0.1
383	0.1	0.7
384	0.3	2.6
385	0.6	6.5
386	1	13.8
387	1.4	26

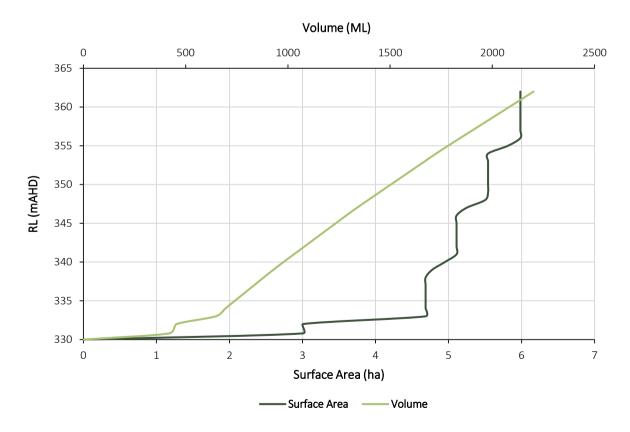




Pit 3

RL (mAHD)	Area (ha)	Volume (ML)
330	0.0	0.0
331	3.0	419.4
332	3.0	455.5
333	4.7	646.8
334	4.7	693.7
335	4.7	740.6
336	4.7	787.4
337	4.7	834.3
338	4.7	881.2
339	4.8	928.9
340	4.9	978.4
341	5.1	1029.5
342	5.1	1080.6
343	5.1	1131.7
344	5.1	1182.8
345	5.1	1233.9
346	5.1	1285.0

RL (mAHD)	Area (ha)	Volume (ML)
347	5.2	1,337.5
348	5.5	1,392.5
349	5.5	1,447.9
350	5.5	1,503.3
351	5.5	1,558.8
352	5.5	1,614.2
353	5.5	1,669.6
354	5.5	1,725.0
355	5.8	1,783.1
356	6.0	1,843.0
357	6.0	1,902.8
358	6.0	1,962.7
359	6.0	2,022.5
360	6.0	2,082.3
361	6.0	2,142.2
362	6.0	2,202.0

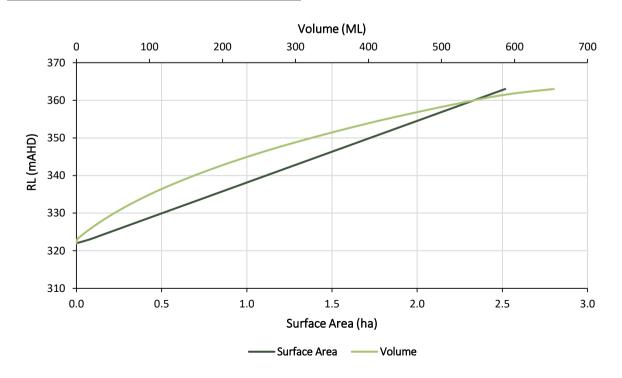




Pit 4

RL (mAHD)	Area (ha)	Volume (ML)
322	0.00	0.0
323	0.08	1.0
324	0.14	6.6
325	0.20	13.2
326	0.26	20.1
327	0.32	27.5
328	0.38	35.3
329	0.44	43.4
330	0.50	51.9
331	0.57	60.8
332	0.63	70.2
333	0.69	79.9
334	0.75	90.1
335	0.81	100.8
336	0.87	111.9
337	0.93	123.5
338	0.99	135.7
339	1.05	148.3
340	1.11	161.5
341	1.18	175.1
342	1.24	189.1
343	1.30	203.7
344	1.36	218.9

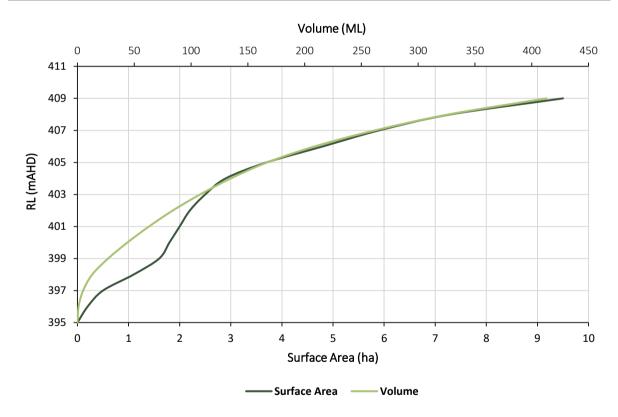
RL (mAHD)	Area (ha)	Volume (ML)
345	1.42	234.5
346	1.48	250.7
347	1.54	267.5
348	1.60	285.0
349	1.66	303.3
350	1.72	322.0
351	1.79	341.3
352	1.85	361.1
353	1.91	381.4
354	1.97	402.3
355	2.03	424.0
356	2.09	446.8
357	2.15	470.3
358	2.21	494.4
359	2.27	519.2
360	2.33	544.9
361	2.40	571.7
362	2.46	606.3
363	2.52	653.8
364	2.58	715.5
365	2.64	786.1
366	2.70	866.7





# Pit 5 South

RL (mAHD)	Area (ha)	Volume (ML)
395	0	0
396	0.2	1
397	0.5	5
398	1.1	13
399	1.6	27
400	1.8	44
401	2	63
402	2.2	84
403	2.5	108
404	2.9	135
405	3.7	167
406	4.8	209
407	5.9	262
408	7.3	327
409	9.5	413

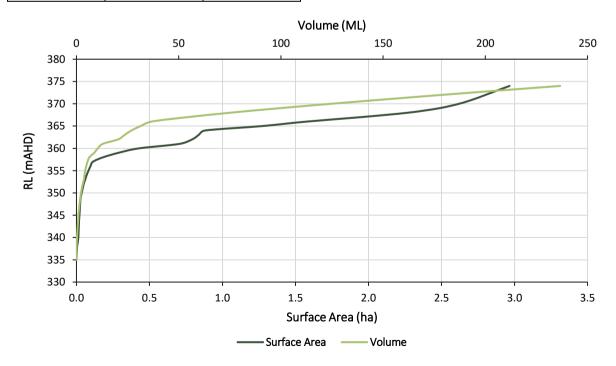




# Pit 5 North

RL (mAHD)	Area (ha)	Volume (ML)
335	0.00	0.00
336	0.00	0.01
337	0.00	0.03
338	0.00	0.07
339	0.01	0.14
340	0.01	0.25
341	0.01	0.38
342	0.02	0.53
343	0.02	0.69
344	0.02	0.87
345	0.02	0.94
346	0.02	1.16
347	0.02	1.39
348	0.03	1.64
349	0.03	1.92
350	0.04	2.24
351	0.04	2.63
352	0.05	3.09
353	0.06	3.64
354	0.07	3.93
355	0.08	4.29
356	0.10	4.79
357	0.11	5.40
358	0.18	6.34

RL (mAHD)	Area (ha)	Volume (ML)
359	0.29	8.68
360	0.43	10.43
361	0.71	12.85
362	0.79	20.34
363	0.84	23.55
364	0.88	26.83
365	1.27	31.34
366	1.57	36.71
367	1.95	54.29
368	2.27	75.36
369	2.48	99.12
370	2.62	124.64
371	2.72	151.33
372	2.80	178.94
373	2.88	207.37
374	2.97	236.61
375	3.05	266.71
376	3.15	297.71
377	3.24	329.67
378	3.35	362.64
379	3.46	396.68
380	3.58	431.87
381	3.71	468.30



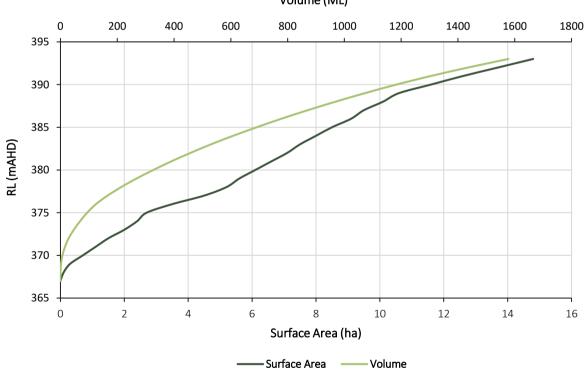


Pit 6

RL (mAHD)	Area (ha)	Volume (ML)
367	0	0
368	0.1	0.2
369	0.3	2
370	0.7	7
371	1.1	16
372	1.5	29
373	2	47
374	2.4	69
375	2.7	94
376	3.5	125
377	4.5	166
378	5.2	214
379	5.6	268
380	6.1	327

RL (mAHD)	Area (ha)	Volume (ML)
381	6.6	390
382	7.1	458
383	7.5	530
384	8	608
385	8.5	691
386	9.1	778
387	9.5	871
388	10.1	969
389	10.6	1073
390	11.6	1183
391	12.6	1304
392	13.7	1436
393	14.8	1578





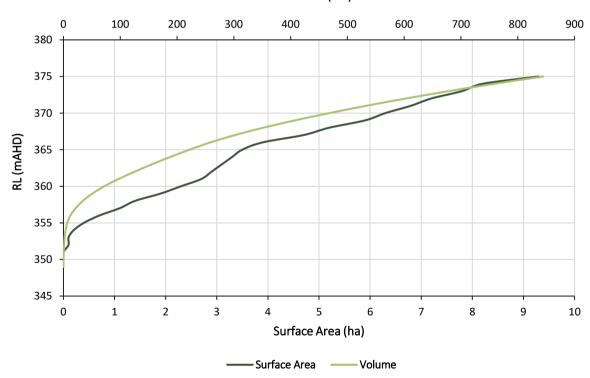


Pit 7

RL (mAHD)	Area (ha)	Volume (ML)
349	0	0
350	0	0.2
351	0	0.5
352	0.1	1.1
353	0.1	2
354	0.2	3.7
355	0.4	6.8
356	0.7	12.1
357	1.1	21.6
358	1.4	34.1
359	1.9	50.9
360	2.3	71.8
361	2.7	97.1
362	2.9	125.6

RL (mAHD)	Area (ha)	Volume (ML)
363	3.1	155.8
364	3.3	188
365	3.5	222.2
366	3.9	259
367	4.7	302.3
368	5.2	352
369	5.9	407.6
370	6.3	468.3
371	6.8	533.7
372	7.2	603.7
373	7.8	678.7
374	8.2	758.5
375	9.3	844.6

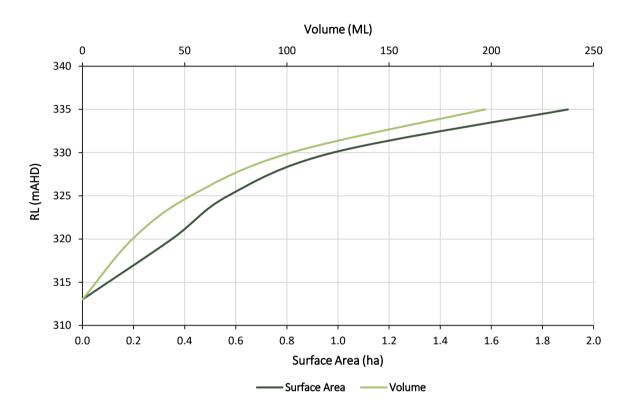
#### Volume (ML)





Pit 8

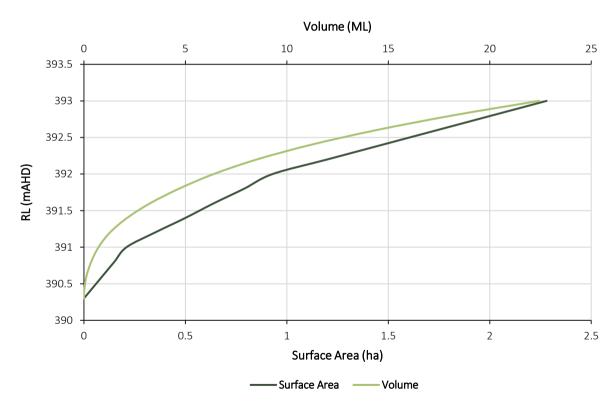
RL (mAHD)	Area (ha)	Volume (ML)
313	0.0	0
320	0.4	25
325	0.6	53
330	1.0	102
335	1.9	197





# Pit 5 FP Dam

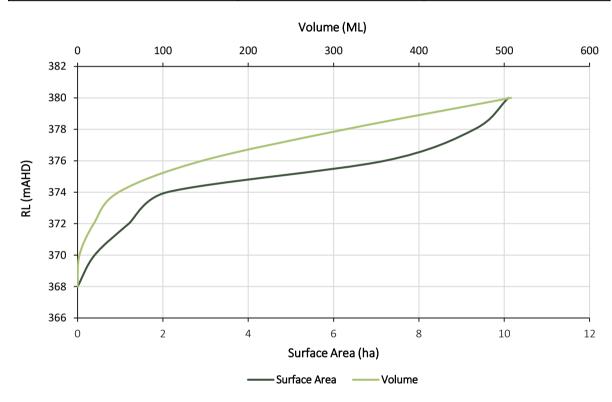
RL (mAHD)	Area (ha)	Volume (ML)
390.3	0	0
390.4	0.03	0.01
390.6	0.09	0.12
390.8	0.15	0.36
391	0.21	0.72
391.2	0.35	1.28
391.4	0.5	2.13
391.6	0.64	3.26
391.8	0.79	4.69
392	0.93	6.4
392.2	1.2	8.53
392.4	1.47	11.19
392.6	0.74	14.4
392.8	2.01	18.15
393	2.28	22.43





# Ed's Lake

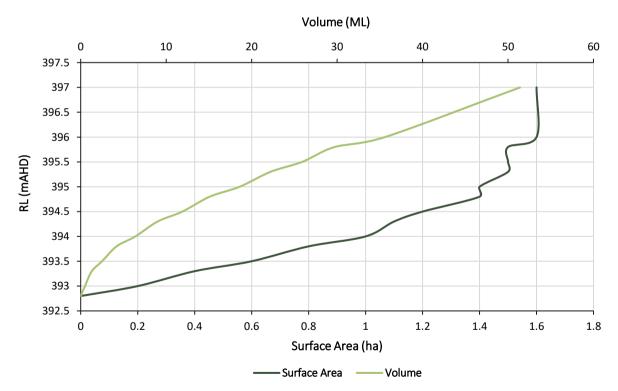
RL (mAHD)	Area (ha)	Volume (ML)
368	0	0
370	0.4	2.2
372	1.2	19
374	2.1	48
376	7.2	146
378	9.3	314
380	10.1	508





# **CWD**

RL (mAHD)	Area (ha)	Volume (ML)
392.8	0	0
393	0.2	0.5
393.3	0.4	1.3
393.5	0.6	2.5
393.8	0.8	4.2
394	1	6.4
394.3	1.1	9
394.5	1.2	11.9
394.8	1.4	15.1
395	1.4	18.6
395.3	1.5	22.2
395.5	1.5	25.9
395.8	1.5	29.7
396	1.6	35.6
397	1.6	51.4

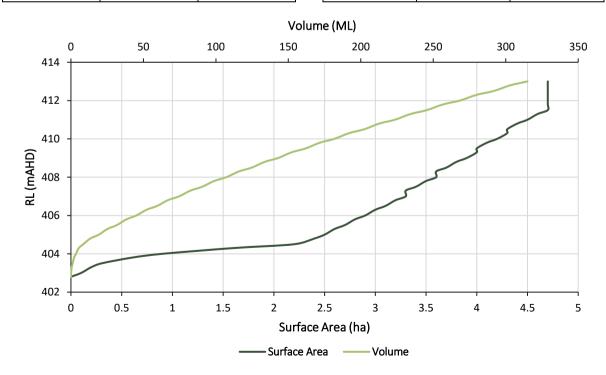




# **RWD**

RL (mAHD)	Area (ha)	Volume (ML)
402.8	0	0
403	0.1	0.1
403.3	0.2	0.5
403.5	0.3	1.2
403.8	0.6	2.1
404	0.9	3.5
404.3	1.6	5.4
404.5	2.2	8.5
404.8	2.4	13.4
405	2.5	19.2
405.3	2.6	25.3
405.5	2.7	31.7
405.8	2.8	38.3
406	2.9	45.1
406.3	3	52.2
406.5	3.1	59.5
406.8	3.2	66.9
407	3.3	74.6
407.3	3.3	82.5
407.5	3.4	90.5
407.8	3.5	98.8

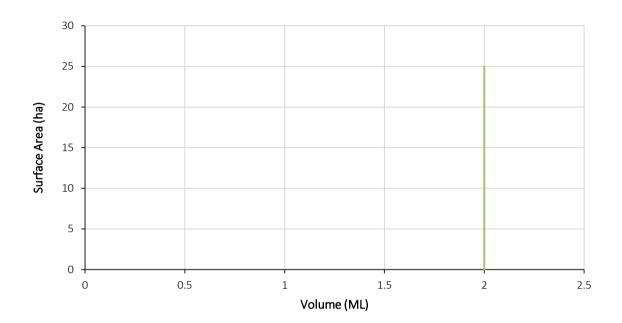
RL (mAHD)	Area (ha)	Volume (ML)
408	3.6	107.2
408.3	3.6	115.9
408.5	3.7	124.7
408.8	3.8	133.7
409	3.9	142.9
409.3	4	152.3
409.5	4	161.9
409.8	4.1	171.7
410	4.2	181.7
410.3	4.3	191.9
410.5	4.3	202.3
410.8	4.4	212.8
411	4.5	223.5
411.3	4.6	234.4
411.5	4.7	245.5
411.8	4.7	256.8
412	4.7	268.3
412.3	4.7	280
412.5	4.7	291.7
412.8	4.7	303.3
413	4.7	315





# Pit 8 CWD

Area (ha)	Volume (ML)
2	0
2	25

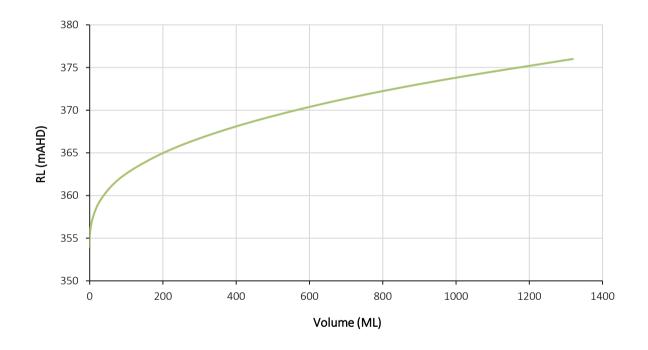




# Pit 1 Spoil Aquifer (20% Porosity)

RL (mAHD)	Volume (ML)
354	0
355	0.01
356	2
357	6
358	13
359	23
360	38
361	57
362	82
363	115
364	155
365	201

RL (mAHD)	Volume (ML)
366	256
367	320
368	392
369	473
370	563
371	662
372	772
373	893
374	1026
375	1171
376	1319

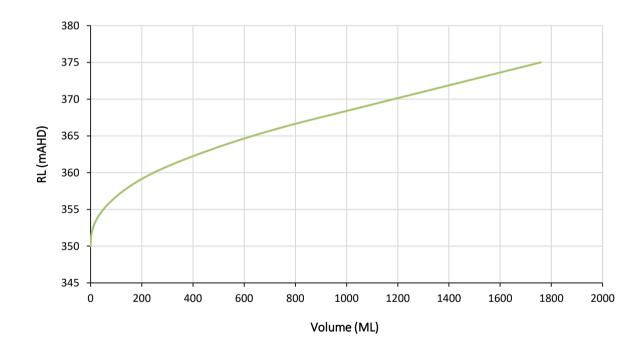




# Pit 2 Spoil Aquifer (20% Porosity)

RL (mAHD)	Volume (ML)
350	0
351	1.2
352	5.8
353	14.8
354	29.3
355	50.1
356	76.8
357	108.7
358	147.1
359	193
360	246.6
361	310.5
362	381.9

RL (mAHD)	Volume (ML)
363	459.6
364	542.7
365	633.3
366	31.6
367	839.3
368	954.1
369	1068.9
370	1183.8
371	1298.6
372	1413.5
373	1528.3
374	1643.2
375	1758.1

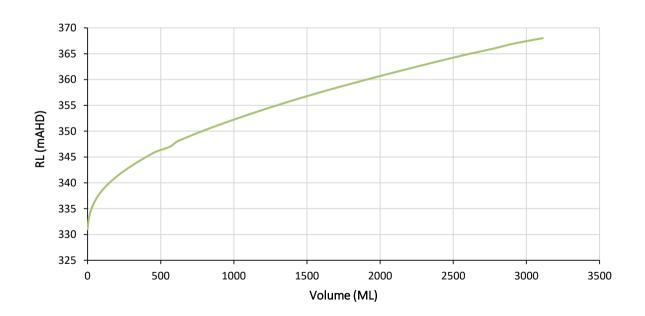




# Pit 4 Spoil Aquifer (10% Porosity)

RL (mAHD)	Volume (ML)
331	0
332	3
333	8.3
334	15.8
335	27.4
336	42.4
337	60.9
338	84.6
339	114.1
340	149.3
341	189.4
342	234.7
343	285.6
344	341
345	401
346	465.7
347	563.3
348	612.9
349	695

RL (mAHD)	Volume (ML)
350	783.4
351	877.6
352	976.2
353	1078.8
354	1184.7
355	1294.4
356	1408.1
357	1526.3
358	1649.9
359	1778
360	1909.5
361	2044
362	2181.4
363	2321.7
364	2466.1
365	2614.9
366	276.8
367	2920.7
368	3111.3





# Pit 5 Spoil Aquifer (10% Porosity)

RL (mAHD)	Volume (ML)
351	0
352	3.5
353	10
354	21.5
355	38.5
356	60.5
357	88
358	123.5
359	167
360	219
361	279.5
362	347
363	421

RL (mAHD)	Volume (ML)
364	501
365	588.5
366	684.5
367	788.5
368	901
369	1021.5
370	1150
371	1285
372	1426.5
373	1575
374	1729.5
375	1889.5
376	2048.5

